



## Construction Specification 004 Concrete

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### A. Scope

The work shall consist of furnishing, forming, placing, consolidating, finishing, and curing Portland cement concrete and the furnishing and placing of steel reinforcement or other appurtenances as required on the construction drawings. All materials, test procedures, and admixtures shall meet the requirements of the latest edition of the applicable ASTM designation.

Failure to meet any requirements contained in this specification may be cause for rejection of the concrete or delay of placement.

### B. Definitions

The following definitions are provided for the purpose of this specification. The words that are defined in this section are italicized the first time they are used in the text.

1. *Batch delivery ticket* refers to the form showing the total weights of all the ingredients used to mix the contents of the rotating drum mixer (total weights of all ingredients on the load) and other job pertinent information.
2. *Consolidating* refers to the process of reducing the volume of entrapped air in a fresh cementitious mixture, usually accomplished by inputting mechanical energy.
3. *Construction joints* are those joints where two successive placements of concrete meet, through which reinforcement is continuous and bond is required between the two pours.
4. *Finishing* refers to the process of treating surfaces of fresh or recently placed concrete or mortar to produce desired appearance and service.
5. *Firm* refers to the condition of the subgrade where it is not significantly displaced or deformed by foot traffic during construction and is able to properly support reinforcement chairs.
6. *Flatwork* refers to concrete slabs poured on slopes flatter than 5:1 (Horizontal:Vertical).
7. *Form release agent* refers to commercially manufactured formwork release agents that prevent formwork absorption of moisture, prevent bond with concrete, and do not stain the concrete surfaces.
8. *Formed surfaces* are those that require a temporary structure or mold for the support of concrete while it is setting and gaining sufficient strength to be self-supporting, such as walls or poured-in-place tank lids.
9. *Hand tamping* refers to the operation of consolidating freshly placed concrete by hand-held implements.
10. *Honeycomb* refers to voids left in concrete due to failure of the mortar to effectively fill the spaces among coarse aggregate particles.
11. *Jitterbug* refers to a grate tamper for pushing coarse aggregate slightly below the surface of a slab to facilitate finishing.
12. *Liquid-containment concrete* refers to concrete applications using specific placement and finishing techniques, and design features to minimize the loss of liquids.
13. *Manufacturer* refers to the producer/supplier of the ready-mixed concrete.
14. *Mesh roller* refers to a finishing tool consisting of a rolling drum attached to a handle, of which the surface of the drum is made of mesh, sometimes used for rolling over the surface of fresh concrete to embed coarse aggregate.
15. *Rock pocket* refers to a porous, mortar-deficient portion of hardened concrete consisting primarily of coarse aggregate and open voids; caused by leakage of mortar from the form, separation (segregation) during placement, or insufficient consolidation.
16. *Sloped slabs* refers to concrete slabs poured on slopes of 5:1 (Horizontal:Vertical) or steeper.
17. *Technician* refers to an individual trained in specific technical processes, and may include an

engineer, government agency representative, private sector technical service provider, qualified independent third-party quality assurance inspector, or a similar person that is primarily responsible for the project quality assurance.

18. *Ternary mix* is a mixture using three cementitious materials, such as Portland cement, fly ash, and ground granulated blast-furnace slag (slag).
19. *Top bars* are horizontal reinforcements placed such that more than 12 inches of fresh concrete is cast below the reinforcing bar (such as horizontal wall bars).
20. *Vibration* refers to mechanical energetic agitation of freshly mixed concrete during placement by mechanical devices, either pneumatic or electric, that create vibratory impulses of moderately high frequency to assist in consolidating the concrete.
  - a. Internal vibration employs one or more vibrating elements that can be inserted into the fresh concrete at selected locations.
  - b. Surface vibration employs a portable horizontal platform on which a vibrating element is mounted.
21. *Water-cement ratio (w/c)* is the ratio of the weight of free water (excluding that absorbed by the aggregates) to the weight of Portland cement in a concrete mix expressed as a decimal.
22. *Water-cementitious material ratio (w/cm)* is the ratio of the weight of free water (excluding that absorbed by the aggregates) to the weight of cementitious material (fly ash, Portland cement, and slag) in a concrete mix expressed as a decimal.

### C. Materials

The Contractor shall provide test data, independent laboratory reports, or other evidence from the concrete manufacturer showing that all materials meet the requirements of this specification. All materials proposed for use shall be approved by the Technician.

1. Portland cement shall conform to ASTM C 150 and shall be Type I, II, or III.
2. Fine aggregate shall conform to ASTM C 33 and be composed of clean, uncoated grains of material. Refer to the fine aggregate gradation table in Section D of this specification.
3. Coarse aggregates shall be gravel or crushed stone conforming to ASTM C 33 and be clean, hard, durable, and free from clay or coating of any character. Refer to the coarse aggregate gradation table in Section D of this specification.
4. Water shall be clean and free from injurious amounts of oil, salt, acid, alkali, organic matter, or other deleterious substances.
5. Air entraining agent shall conform to ASTM C 260.
6. Pozzolan (fly ash) shall conform to ASTM C 618, Class C or F. The loss of ignition shall not exceed 2 percent for Class C and 6 percent for Class F.
7. Ground granulated blast furnace (GGBF) slag shall conform to ASTM C 989.
8. Chemical admixtures shall be used in strict compliance with the manufacturer's recommendations, conform to ASTM C 494, and may be the following types:
  - a. Type A - Water-reducing admixtures.
  - b. Type B - Retarding admixtures.
  - c. Type C - Accelerating admixtures.
  - d. Type D - Water-reducing and retarding admixtures.
  - e. Type E - Water-reducing and accelerating admixtures.
  - f. Type F - Water-reducing, high range admixtures (superplasticizers).
  - g. Type G - Water-reducing, high range, and retarding admixtures (superplasticizers).
  - h. Type S - Specific performance admixtures
    - If Type C or E is used, the manufacturer shall provide the Technician a product data sheet

verifying that the product is a non-chloride accelerator.

- If Type S is used the manufacturer shall provide the Technician a report stating the specific performance characteristic(s) of the admixture and data to substantiate the performance characteristic(s).
  - Calcium chloride or admixtures containing chloride ions other than from impurities in admixture ingredients shall not be used.
9. Deformed reinforcing bars shall be free from loose rust, oil, grease, paint, or other deleterious matter. Steel bars for concrete reinforcement shall meet the requirements of ASTM A 615. The steel shall be deformed Grade 40 or Grade 60 billet-steel bars as noted on the plans.
  10. Deformed welded wire reinforcement (WWR) shall conform to the requirements of ASTM A 1064 and shall be furnished in flat sheets and shall be size D4 or larger as indicated on the plans. This material may only be used for non-structural elements such as slabs on grade. Spacing of welded intersections shall not exceed 16 inches.
  11. Curing compound shall be a liquid membrane-forming compound suitable for spraying on the concrete surface. The curing compound shall meet the requirements of ASTM C 309, Type 2 (white pigmented).

#### **D. Design of the Concrete Mix**

No less than seven (7) days prior to the start of concrete placement the Contractor is responsible for submitting documentation of the proposed design mix to the Technician. The Contractor is responsible for providing a mix with the minimum required 28-day compressive strength in the construction plan and meet the following:

1. The water-cement (w/c) or the water-cementitious material (w/cm) ratio shall not exceed 0.45 for all concrete construction.
2. The cementitious material required shall be a minimum of 564 pounds per cubic yard of concrete.
  - a. The cementitious material may include a maximum of 25 percent (by weight) of fly ash or a maximum of 30 percent (by weight) of ground granulated blast-furnace (GGBF) slag. The remaining cementitious materials shall be Portland cement.
  - b. Mixes containing both fly ash and GGBF slag shall not exceed 30 percent in combination (ternary mix) and no more than 25 percent shall be fly ash. The remaining cementitious materials shall be Portland cement.
3. The air content (by volume) shall be 6 percent of the volume of the concrete.
4. The maximum (not to exceed) slump, with the use of water reducers, shall be 5 inches  $\pm$  0.25 inches.
5. The maximum (not to exceed) slump, with the use of superplasticizers, shall be 8 inches  $\pm$  0.25 inches.
6. The fine aggregate saturated surface dry weight shall be 30-45 percent of the total saturated surface dry weight of the combined coarse and fine aggregates. The well-graded fine aggregate shall conform to the following ASTM C 33 or Wisconsin DOT gradation requirements shown below:

**Fine Aggregate Gradation**

| Sieve Size               | Percent Passing By Weight |        |
|--------------------------|---------------------------|--------|
|                          | ASTM C 33                 | WI DOT |
| $\frac{3}{8}$ " (9.5 mm) | 100                       | 100    |
| No. 4 (4.75 mm)          | 95-100                    | 90-100 |
| No. 8 (2.36 mm)          | 80-100                    | ---    |
| No. 16 (1.18 mm)         | 50-85                     | 45-85  |
| No. 30 (600 $\mu$ m)     | 25-60                     | ---    |
| No. 50 (300 $\mu$ m)     | 5-30                      | 5-30   |
| No. 100 (150 $\mu$ m)    | 0-10                      | 0-10   |
| No. 200 (75 $\mu$ m)     | 0-5                       | 0-3.5  |

7. The well graded coarse aggregate shall conform to the following ASTM C 33 gradation requirements for size number 67 aggregate shown below:

**Coarse Aggregate Gradation**

| Sieve Size                | Percent Passing By Weight |
|---------------------------|---------------------------|
| 1" (25.0 mm)              | 100                       |
| $\frac{3}{4}$ " (19.0 mm) | 90-100                    |
| $\frac{3}{8}$ " (9.5 mm)  | 20-55                     |
| No. 4 (4.75 mm)           | 0-10                      |
| No. 8 (2.36 mm)           | 0-5                       |
| No. 200 (75 $\mu$ m)      | 0-1.5                     |

**E. Mixing**

- Ready-mixed concrete shall be in accordance with ASTM C 94 for ordering (OPTION C, Minimum Cement Content), batching, mixing, and transporting.
- Batching Tolerances (maximum w/c or w/cm ratio shall not exceed 0.45):
  - Cementitious Material: The weight of the cementitious material shall be within plus or minus 1 percent ( $\pm 1\%$ ) of the required weight of the cementitious material.
  - Admixtures: The admixtures shall be within plus or minus 3 percent ( $\pm 3\%$ ) of the required weight or volume for each specific admixture.
  - Mixing Water: The water added to the batch, including free water on the aggregates, shall be measured by weight or volume to an accuracy of 1 percent of the required total mixing water.

Added ice shall be measured by weight.

- d. Aggregate: The weight of the fine and coarse aggregate shall be within plus or minus 2 percent ( $\pm 2\%$ ) of the required weight.
  - e. Air: The air content (by volume) shall be  $6 \pm 1.5$  percent of the volume of the concrete at the location and time of placement.
- 3. Concrete shall be uniform and thoroughly mixed when delivered to the forms.
  - 4. The water-cement (w/c) ratio or water-cementitious material (w/cm) shall not exceed 0.45 at any time, including the addition of water at the site.
  - 5. The concrete shall be batched and mixed such that the temperature of the concrete at time of placement shall not be less than 55 degrees Fahrenheit or, at no time during its production or transportation more than 90 degrees Fahrenheit.

#### **F. Batch Delivery Ticket Information**

- 1. The Contractor shall obtain from the manufacturer a batch delivery ticket for each load of concrete before unloading at the site. Any concrete load delivered without a batch delivery ticket containing all the following information shall not be allowed to be discharged in any part of the construction project covered under this specification.
- 2. The following minimum information shall be included on the batch delivery ticket.
  - a. Job-pertinent information
    - i. Name of concrete manufacturer and batch plant
    - ii. Name of purchaser and job location
    - iii. Date of delivery
    - iv. Truck number
    - v. Amount of concrete delivered
    - vi. Time loaded or time of first mixing of cement and aggregates
  - b. Ingredients used to mix the batch
    - i. Mixing water added as free water
    - ii. Percent moisture, or weight of water contained on the aggregates
    - iii. Percent absorption, or weight of water absorbed by the aggregates
    - iv. Type and amount of cementitious materials
    - v. Type and amount of admixtures
    - vi. Weights of fine and coarse aggregates
  - c. The Contractor is responsible for adding the following information:
    - i. Volume of water added by the receiver of the concrete
    - ii. Time the concrete arrived at the site
    - iii. Time the concrete was completely unloaded
- 3. Upon completion of the concrete placement, copies of all batch delivery tickets shall be provided to the Technician.

#### **G. Placement of Subgrade, Forms, and Reinforcing Steel**

- 1. Subgrade
  - a. The site shall be graded to the dimensions and elevations as specified in the construction plans.
    - All surfaces shall be firm and damp prior to placing concrete.
    - Concrete shall not be placed on mud, dried earth, uncompacted fill, frozen subgrade, or in standing water.

- The use of plastic sheeting beneath the concrete is not permitted except for a designed vapor barrier in an enclosed building.

## 2. Forms

- a. The forms, associated bracing, and stakes shall be substantial, unyielding, and constructed so that the finished concrete will conform to the specified dimensions and contours.
  - Forms shall be mortar tight.
  - Forms shall be coated with a form release agent before being set into place.
  - Form release agent shall not come in contact with the steel reinforcement, waterstop, or with hardened concrete against which fresh concrete is to be placed.
  - For structures which are to be store liquids, form ties shall be used that permit their removal to a depth of at least ½ inch.
  - Concrete joints shall be placed at locations and be of the type shown on the construction drawings.

## 3. Reinforcing Steel

Reinforcement shall be accurately placed as shown on the drawings and secured in position in a manner that will prevent its displacement during the placement of concrete.

- a. Tolerances - The following tolerances will be allowed in the placement of reinforcement:
  - Where 1½ inches clear distance is shown between reinforcing steel and forms, or embedded objects, allowable clear distance is 1⅞ to 1½ inches.
  - Where 2 inches clear distance is shown between reinforcing steel and forms, allowable clear distance is 1⅝ to 2 inches.
  - Where 3 inches clear distance is shown between reinforcing steel and earth or forms, allowable clear distance is 2½ to 3 inches. Over-excavation backfilled with concrete shall not be considered as clear distance.
  - The maximum variation from the reinforcing steel spacing shown, shall be 1/12 of the spacing, without a reduction in the amount of reinforcing steel specified.
  - The ends of all reinforcing steel shall be covered with at least 1½ inches of concrete, with an allowable minimum distance of 1⅞ inches.
- b. Reinforcement Support - Holding steel reinforcement in position with temporary supports is not permitted. Tack welding of bars is not permitted.
  - Steel chairs, hangers, spacers; coated steel chairs, hangers, spacers; or plastic chairs, hangers, spacers may be used as supports.
  - Precast concrete chairs may be used as supports providing the chairs are manufactured from concrete equal in compressive strength to the concrete being placed.
  - Reinforcement shall be supported at a minimum as follows:
    - Deformed reinforcing bars for flatwork and sloped slabs shall be supported by a minimum of 1 support chair every 4 feet in each direction. Reinforcement shall not deflect or sag between supports. Deformed reinforcing bars shall be tied at every other rebar intersection or as approved by the Technician.
    - Deformed welded wire reinforcement (WWR) shall be supported no further than as indicated in the table below.
    - When two layers of deformed reinforcing bars or deformed welded wire reinforcement are used for wall footings, flatwork and sloped slabs, the bottom layer may be supported by precast concrete chairs. The upper layer must be supported by metal chairs, metal spacers, plastic spacers, or rebar with legs tied to the lower mat and supporting the upper layer of reinforcing bars.



## WWR Support

| Welded Wire Reinforced Size <sup>(1)</sup> | Welded Wire Spacing | Maximum Support Spacing in Each Direction <sup>(2)</sup> , feet |
|--|---------------------|---|
| D9 or larger                               | 12 inches or more   | 4 to 6 feet   |
| D5 to D8                                   | 12 inches or more   | 3 to 4 feet   |
| D9 or larger                               | Less than 12 inches | 3 to 4 feet   |
| D4 to D8                                   | Less than 12 inches | 2 to 3 feet   |

Notes:

(1) "D" is the standard designation for deformed wire.

(2) Support spacing shall be adequate to support all loads, including construction personnel and equipment. If excessive deflections occur, closer support spacing is required.

- c. Flatwork reinforcement may be driven on prior to placement of supports if both of the following conditions are met:
  - The subgrade is firm so that minimal displacement is made by equipment. If significant displacement occurs, the steel shall be removed, the subgrade regraded and compacted before steel and concrete placement.
  - The reinforcing steel is not deformed by the equipment. If the steel is deformed, it shall be replaced before concrete placement.
- d. Steel tying to protruding steel from a previous pour or form construction for new concrete that will be in contact with previously poured concrete shall not be started until the previously poured concrete has cured a minimum of 12 hours.
- e. Reinforcement Splice Lengths and Bend diameters:
  - Deformed reinforcing bars
    - Bend diameter: 6 bar diameters for #3 through #8 bar sizes and 8 bar diameters for larger bars. Reinforcing bars shall not be heated to facilitate bending.
    - Splice Length: The minimum splice lengths in the table below are for concrete designed with a 28-day compressive strength of 3,500 psi. (NRCS standard wall designs) Other higher concrete design strengths and reinforcement grades require different splice lengths (typically shorter) in accordance with ACI 318.
    - Deformed reinforcing bars shall not be spliced by welding. All lap splices shall be adequately tied together to firmly hold the reinforcement in position to maintain the proper splice length.

### Minimum Splice Lengths <sup>Note 1</sup>

|   | Grade 40                             | Grade 60                             |
|---|--------------------------------------|--------------------------------------|
| #3 through #6 bars<br><i>Top bars</i><br>all other bars | 27 bar diameters<br>21 bar diameters | 41 bar diameters<br>32 bar diameters |
| #7 and larger bars<br><i>Top bars</i><br>all other bars | 34 bar diameters<br>26 bar diameters | 51 bar diameters<br>40 bar diameters |

<sup>Note 1</sup>: Splice lengths shall be the greater of that indicated in the Table or 12-inches.

- Deformed welded wire reinforcement (WWR) - Splice length shall be in accordance with the requirements of ACI 318-08 or ACI 318-11 Part 12.18. Deformed welded wire reinforcement shall not be spliced by welding. All lap splices shall be tied to firmly hold the reinforcement in position to maintain the proper splice length.

## **H. Delivering, Placing, Consolidating, and Finishing Concrete**

1. The Contractor shall notify the Technician of the proposed method of placement, consolidation, and finishing of the concrete at least seven (7) days prior to the start of concrete placement. The Contractor shall furnish the Technician a record of daily data including:
  - a. Ambient temperature
  - b. Relative humidity
  - c. Wind velocity
2. General
  - a. Prior to placement of concrete, the forms and subgrade shall be free of chips, sawdust, debris, water, ice, snow, extraneous oil, mortar, or other harmful substances or coatings. Any oil on the reinforcing steel or other surfaces required to be bonded to the concrete shall be removed. Concrete shall not be placed until the subgrade, forms, waterstop, and steel reinforcement have been inspected and accepted by the Technician. Any deficiencies shall be corrected before the concrete is delivered for placement. Forms, reinforcing steel, and subgrade shall be moistened prior to placing concrete.
3. Delivery
  - a. Concrete shall be delivered to the site and discharged into the forms within 1½ hours after the introduction of the mixing water to the cement and aggregates, or when a superplasticizer is used, the manufacturer's recommended time limit for discharge after addition shall apply. The 1½ hour time may be extended if the concrete is of a slump that it can be placed, consolidated, and finished without the addition of water to the batch. Upon arrival at the job site, addition of water will be allowed to adjust the slump, provided such addition does not exceed the water-cement (w/c) ratio or water-cementitious material ratio (w/cm). Final placement of the batch shall begin immediately after mixing of the added water is completed.
  - b. Additional superplasticizer shall not be added to the concrete mix after discharge of the concrete at the job site has commenced.
4. Placement
  - a. The slump of the placed concrete shall not exceed the maximum slump of 5 inches  $\pm$  0.25 inches with the use of water reducers.
  - b. The slump of the placed concrete shall not exceed the maximum slump of 8 inches  $\pm$  0.25 inches with the use of superplasticizers.
  - c. Concrete shall be deposited as closely as possible to its final position. Concrete shall be worked into the corners and angles of the forms and around all reinforcement and embedded items in a manner to prevent segregation of aggregates. All placement shall be done in a manner that prevents incorporation of subgrade material into the concrete.
  - d. Methods for placing concrete on sloped slabs shall only include chutes, pumps, conveyors, wheelbarrows, or similar means of directly depositing concrete as near as possible to its final position. Placement of concrete by other methods where concrete is deposited upslope and flows to its final position downslope (commonly called "lava flow", "glacial pours", etc.) shall not be permitted.
  - e. Concrete shall not be dropped more than 6 feet vertically unless suitable equipment is used to prevent segregation. Concrete containing superplasticizer shall not be dropped more than 12 feet vertically and shall not be placed in lifts exceeding 6 feet in depth. Non-superplasticized concrete shall be placed in forms in horizontal layers not more than 24 inches deep. Each layer shall be thoroughly consolidated before the next is placed, at a rate such that previously placed concrete has not yet set when the next layer of concrete is placed upon it.



## 5. Consolidation

- a. All concrete required to be consolidated with internal type mechanical vibrator shall be capable of transmitting vibration to the concrete at frequencies not less than 8,000 impulses per minute, unless otherwise specified or approved before placement.
  - Vibration shall compact the concrete and bring it into intimate contact with the forms, reinforcing steel, and other embedded items while removing voids and pockets of entrapped air. The location, insertion, duration, and removal of the vibrators shall be such that maximum consolidation of the concrete is achieved without causing segregation of the mortar and coarse aggregate or causing water or cement paste to flush to the surface.
  - Vibration shall be supplemented by spading, rodding, and hand tamping as necessary to ensure smooth and dense concrete along the form surface, in corners, and around embedded items. The contractor shall provide a sufficient number of vibrators to properly consolidate the concrete immediately after it is placed. Placement rate will be restricted if an inadequate number of vibrators are available.
  - The use of vibrators to transport concrete in the forms, slabs or conveying equipment will not be permitted.
- b. Formed Surfaces
  - All concrete walls shall be vibrated.
  - Immediately after the concrete is placed in the forms, it shall be consolidated by internal vibration or hand tamping as necessary to insure dense concrete. Vibration shall be applied to the freshly deposited concrete by rapidly inserting the vibrator and slowly, in an up and down motion, removing the vibrator at points uniformly spaced at not more than 1.5 times the radius of the area visibly effected by vibration. Generally, this is at 5 to 10 seconds per foot on 14-inch spacings or less. The area visibly effected by the vibrator shall overlap the adjacent, just vibrated area. The vibrator shall extend vertically into the previously placed layer of fresh concrete by at least 6 inches at all points. Concrete supplied with superplasticizer shall be placed with a minimum amount of vibrating and finishing effort. Vibration shall not be applied directly to the reinforcement steel or the forms, nor to concrete which has hardened to the degree that it does not become plastic when vibrated. Each pour shall be consolidated to insure a monolithic bond with the preceding pour.
- c. Slabs and footings
  - Immediately after the concrete is placed, it shall be consolidated by hand or mechanical methods as necessary to insure dense concrete.
    - Surface vibrators may be used to consolidate slabs 8 inches and less in thickness. In thin slabs the internal vibrator(s) should be sloped toward the horizontal to allow operations in a fully embedded position, but shall not contact the subgrade.
    - Slabs and footings more than 8 inches thick shall be consolidated with internal vibration and may be augmented through use of a surface vibrator.
  - Surface vibrators include vibrating screeds, plate or grid vibratory tampers, or vibratory roller screeds. (Mesh rollers, jitterbugs, and grate tampers are finishing tools and not consolidation tools.) When the concrete slab is to be consolidated using surface vibration methods, the contractor shall detail how this work is to be performed in writing to the technician for review and approval. This report must be submitted no less than 7 calendar days before placing concrete by this method. It includes equipment selection and specifications.

## 6. Finishing

- a. All screed support devices shall be removed from the concrete or driven down flush with the subgrade prior to finishing.
- b. All formed concrete surfaces shall be true and even, and shall be free of depressions, holes,

projections, bulges, or other defects in the specified surface finish or alignment. All surface defects shall be repaired as stated in the "Form Removal" section of this specification.

- c. All flatwork and sloped slabs shall be worked to a uniform grade, maintaining the specified thickness. Concrete shall be worked to minimize segregation and in a manner that does not adversely affect the structural integrity, durability, or function of the structure. Surfaces shall be free from rock pockets, or honeycomb areas or other harmful irregularities or defects.
- d. Water shall not be sprinkled or added to the surface of the concrete to facilitate finishing. An additional finish shall be applied if specified in the construction plans.
- e. The proposed finished texture (broom, float, mesh roller, trowel, non-slip, etc.) of the concrete surface shall be approved by the Technician.
- f. Evaporation reducer may be used during the finishing operation if approved by the Technician. Curing of the concrete is still required as per Section K, Curing.
- g. If a protective concrete coating is specified on the drawings, the coating manufacturer's recommendations for curing and surface preparation shall be followed.

## **I. Construction Joints**

1. If the concrete sets during placement to the degree that it will not flow and merge with the succeeding pour when tamped or vibrated, the Contractor shall discontinue placing concrete and install a formed construction joint. The Contractor shall be prepared to install unplanned construction joints in the event that there is an interruption of the pour, equipment breakdown, or other problem which makes it necessary to stop placement of concrete at locations other than those previously planned. The reinforcement shall pass through the joint, unless otherwise indicated on the construction plan. Prior to the succeeding pour, the joint surface shall be cleaned to remove all unsatisfactory concrete, laitance, coatings, stains, or debris by one of the following methods:
  - a. The joint surface shall be cleaned to expose the fine aggregate and sound surface mortar, but not so deep as to undercut the edges of coarse aggregate. Cleaning shall be by wire brush, sandblasting, or high pressure air-water cutting after the concrete has gained sufficient strength to prevent displacement of the coarse aggregate. The joint surface shall be washed to remove all loose material after cutting.
  - b. According to methods specified by the person approving the construction plans.
2. The surfaces of all construction joints shall be wetted and standing water removed immediately prior to placement of the new concrete. The new concrete shall be placed directly on the cleaned and washed surface. New concrete shall not be placed until the hardened concrete has cured at least 12 hours. The newly placed concrete shall be consolidated to achieve a good bond with the previously hardened concrete.

## **J. Form Removal and Concrete Repair**

1. Form Removal
  - a. Forms shall be removed without damage to the concrete. Supports shall be removed in a manner that permits the concrete to take the stresses due to its own weight uniformly and gradually. The minimum period from completion of the concrete placement to the removal of the forms shall be based on either strength tests or cumulative times.
    - The strength of the in-place concrete is determined by testing concrete cylinders specifically cast for this purpose and cured adjacent to the member in accordance with the ASTM C 31 methods for determining removal time. Unless otherwise specified, forms supporting the weight of the concrete member may be removed after the concrete strength is 70 percent of that specified for the 28-day compressive strength.
    - The total accumulated time, not necessarily continuous, that the air adjacent to the concrete is above 50 degrees Fahrenheit will be determined by the Contractor and accepted by the Technician. The forms may be removed after the total accumulated time shown in the following table:

## Form Removal

| Forms                                     |                           | Time     |
|---|---------------------------|----------|
| Sides of slabs or beams without waterstop |                           | 12 hours |
| Sides of slabs or beams with waterstop    |                           | 16 hours |
| Undersides of slabs or beams              | Clear Span < 10 feet      | 4 days   |
|   | 10-20 feet                | 7 days   |
|   | > 20 feet                 | 14 days  |
| Sides of walls or columns                 | Height of forms < 20 feet | 24 hours |
|   | > 20 feet                 | 72 hours |

- b. For structures which are not required to store liquid, form ties shall be removed flush with or below the concrete surface. For structures which are to be store liquid, form ties shall be removed to a minimum depth of ½ inch. All cavities or depressions resulting from form tie removal shall be patched in accordance with J.(2)(d).
  - c. Forms shall be removed and the concrete inspected by the Technician before walls are backfilled. Concrete loading shall be in accordance with Section N, Loading New Reinforced Concrete Structures.
2. Repair of Surface Defects (other than tie holes)
- a. Immediately after removal of the forms, concrete which is honeycombed, damaged or otherwise defective as identified by the Technician shall be repaired or replaced by the Contractor. All repairs of surface defects shall be completed prior to the application of curing compound. Repair of surface defects such as honeycombed or otherwise defective concrete shall be made using bonding grout and site mixed Portland cement mortar or other products specifically intended to repair surface defects that are applied in accordance with the manufacturer's recommendations.
  - b. Bonding grout and site mixed Portland cement mortar:
    - Outline the honeycombed or otherwise defective concrete with a ½ to ¾ inch deep saw cut and remove such concrete down to sound concrete. When chipping is necessary, leave chipped edges perpendicular to the surface or slightly undercut. Do not feather edges.
    - Dampen the area to be patched plus another 6 inches around the patch area perimeter.
    - Prepare bonding grout by mixing approximately one part Portland cement and one part fine sand with water to the consistency of thick cream.
    - Thoroughly brush the bonding grout into the surface. When the bond coat begins to lose water sheen, apply repair mortar. Repair mortar is made by mixing 1 part Portland cement to 2½ parts fine sand (approximately finer than the No. 16 sieve size) by damp loose volume. The mortar shall be at a stiff consistency with no more mixing water than is necessary for handling and placing. Mix the repair mortar and manipulate the mortar frequently with a trowel without adding water.
    - Thoroughly consolidate the mortar into place and strike off, leaving the patch slightly higher than the surrounding surface to compensate for shrinkage. Leave the patch undisturbed for 1 hour before finishing. The repair shall be cured as specified Section K, Curing.
  - c. Repair materials other than site mixed Portland cement:
    - Portland cement mortar modified with a latex bonding agent conforming to ASTM C 1059, Type II.
    - Epoxy mortars and epoxy compounds that are moisture-insensitive during application and after curing and that embody an epoxy binder conforming to ASTM C 881. The type, grade,

and class shall be appropriate for the application as specified in ASTM C 881.

- Nonshrink Portland cement grout conforming to ASTM C 1107.
  - Packaged dry concrete repair materials conforming to ASTM C 928.
  - Other products specifically intended to repair surface defects that are applied and cured in accordance with the manufacturer's recommendations.
- d. Repair of Form Tie Holes
- Liquid Containment Concrete Structures – Repair tie holes immediately after formwork removal and prior to the application of curing compound. All cavities or depressions resulting from form tie removal shall be patched with commercially available patching products or site mixed Portland cement repair mortar.
    - Site-mixed Portland cement repair mortar
      - Repair mortar is made by mixing 1-part cement to 2.5-parts fine sand (approximately finer than the No. 16 sieve size) by damp loose volume. Mortar shall be at a stiff consistency with no more mixing water than is necessary for handling and placing. Mix the repair mortar and manipulate the mortar frequently with a trowel without adding water. Clean and dampen tie holes before applying the mortar. Cure in accordance with Section K, Curing.
    - Repair materials other than site mixed Portland cement:
      - All those materials listed in J.2.(c).
      - Other products specifically intended to fill form tie holes for liquid containment applications that are applied and cured in accordance with the manufacturer's recommendations.

## **K. Curing**

1. Concrete shall be cured for a period of at least 7 consecutive days (curing period) after it is placed, except as stated in Section M. Exposed concrete surfaces shall be kept continually wet during the entire curing period or until curing compound is applied.
2. Curing compound shall be applied at the rate and with the proper equipment recommended by the manufacturer. It shall form a uniform, continuous, adherent film that shall not check, crack, or peel and shall be free from pinholes or other imperfections.
3. Curing compound shall not be used at construction joints or other areas that are to be bonded to additional concrete. Surfaces subjected to heavy rainfall or running water within 3 hours after the application of curing compound, or surfaces damaged by subsequent construction operations during the curing period, shall be recoated in the same manner as the original application.
4. Concrete in feed storage areas shall be allowed to cure or be protected from contact with stored feed for a minimum of 28 days.

## **L. Concrete Placement in Hot Weather**

1. Hot weather conditions exist at the time of proposed placement when:
  - a. The rate of evaporation greater than 0.10 lb. /sq. ft. /hr. OR
  - b. Two or more of the following factors are exceeded:
    - Ambient temperature is greater than 80 degrees Fahrenheit
    - Relative humidity is less than 60 percent
    - Wind velocity (average) is greater than 10 mph
2. Concrete surfaces shall not be allowed to dry after placement and during the curing period.
3. Measures to reduce surface moisture loss and rate of cement hydration must be taken to immediately protect and cure the concrete due to rapid drying conditions.
  - a. Plan placement to early morning, late afternoon or evening.
  - b. Use a set-retarding admixture meeting the requirements in Section C when the time between the introduction of the mixing water to the cement and aggregates and discharge exceeds 45

- minutes. The 45 minute time may be extended if the concrete is of a slump that it can be placed, consolidated, and finished without the addition of water to the batch.
- c. Use a fog spray to raise the relative humidity of the ambient air.
  - d. Moist cure the concrete surface as soon as the surfaces are finished and continue for at least 24 hours.
  - e. Use a monomolecular film, or evaporation retarder in accordance with the manufacturers printed instructions.
4. Concrete placement shall be suspended when:
- a. The rate of evaporation is greater than 0.25 lb./sq. ft./hr. OR
  - b. When all of the following factors, as measured at the time of concrete placement are exceeded:
    - The ambient temperature is greater than 80 degrees Fahrenheit,
    - Relative humidity is less than 40 percent, and
    - Wind velocity (average) is greater than 15 mph
$$E = (T_c^{2.5} - R * T_a^{2.5}) (1 + 0.4V) \times 10^{-6}$$

where:

E = evaporation rate, lb. /sq. ft. /hr.

T<sub>c</sub> = concrete temperature, °F

T<sub>a</sub> = air temperature, °F

R = percent relative humidity /100 (decimal form 20% = 0.20)

V = wind velocity, mph
5. Wind speeds at reporting station are taken above the ground surface, so V = average reported wind speeds x 0.66).

#### **M. Concreting in Cold Weather**

1. The following provisions shall apply when the minimum air temperature at the local job site is less than 35 degrees Fahrenheit (the forecasted temperature, which shall be verified with a maximum/minimum thermometer at the start of the morning job shift).
  - a. No concrete shall be placed without the required thermometers at the job site.
  - b. The Contractor shall furnish the Technician a record of daily temperature data including:
    - Outside air maximum and minimum temperatures at the local job site, and
    - Temperatures, of the air adjacent to the surface of the concrete, at several points along the concrete surface for all concrete curing periods.
  - c. When the cement is initially added to the mix, the temperature of the mixing water shall not exceed 100 degrees Fahrenheit nor shall the temperature of the aggregate exceed 100 degrees Fahrenheit.
  - d. The temperature of the concrete at the time of placement shall be not less than 55 degrees Fahrenheit or at no time during its production or transport more than 90 degrees Fahrenheit.
  - e. Placed concrete may be protected by covering, housing, insulating or heating concrete structures.
  - f. The minimum air temperature adjacent to the surface of the concrete shall be maintained above 40 degrees Fahrenheit for a period of at least 7 accumulated days. These 7 days must occur during the first 10 days after the concrete is placed. At no time, during the first 10 days after concrete is placed, shall the minimum air temperature adjacent to the surface of the concrete be less than 32 degrees Fahrenheit unless Type III cement or an approved accelerating admixture is used (see Item (g) below).

- g. The curing period may be reduced from 7 cumulative days to 3 consecutive days when Type III cement or an approved accelerating admixture is used. The accelerating admixture shall be used at the proportions recommended by the manufacturer. The minimum air temperature adjacent to the surface of the concrete shall be maintained above 40 degrees Fahrenheit for the 3 day curing period.
- h. Combustion heaters shall have exhaust flue gases vented out of the concrete protection enclosure. The heat from heaters and ducts shall be directed in such a manner as to not overheat or dry the concrete in localized areas or to dry the exposed concrete surface.
- i. At the end of the curing period, the concrete shall be allowed to cool gradually. The maximum temperature decrease at the concrete surface in a 24-hour period shall not exceed 40 degrees.

#### **N. Loading New Reinforced Concrete Structures**

1. Backfill material shall be the type indicated on the drawings and shall be free of large stones or debris.
2. Compaction within 3 feet of the new structure wall will be by means of small manually directed tamping or vibrating equipment.
3. Days before backfilling:
  - a. The age of concrete shall be at least 14 days prior to backfilling for vertical or near-vertical walls with earth loading on one side only and prior to backfilling for conduits and spillway risers with inside forms removed.
  - b. The age of concrete shall be at least 7 days before any load (including backfill) is applied for walls backfilled on both sides simultaneously and prior to backfilling conduits and spillway risers with inside forms and bracing in-place.
  - c. Loads may be applied to new concrete less than the specified days (7 or 14) after placement when the design strength has been attained and verified through compressive strength testing on cylinders that have been cured on-site under field conditions.

#### **O. Inspection and Testing**

1. The inspection and testing details of this section shall apply when specific concrete tests are required in the construction drawings or quality assurance plan. This testing does not relieve the Contractor of the responsibility to perform the work according to this specification. The Technician shall have free access to the work site and batching to obtain samples.
2. When testing is conducted, the following methods shall be used:

##### **Testing**

| Type of Test                             | Test Method (ASTM Designation) |
|--|--------------------------------|
| Sampling                                 | C 172                          |
| Slump                                    | C 143                          |
| Air Content                              | C 231 or C 173                 |
| Making and Curing Specimens in the Field | C 31                           |
| Obtaining and Testing Drilled Cores      | C 42                           |
| Compressive Strength                     | C 39                           |
| Density (Unit Weight)                    | C 138                          |
| Temperature                              | C 1064                         |



3. The contractor is responsible for determining who is responsible for testing, and providing results to all parties.
4. Compressive strength of the concrete shall be considered satisfactory if test results equal or exceed the 28-day design strength. For each ASTM C 39 strength test, three test specimens shall be made. The test result shall be the average of the compressive strength tests of any two of the three test specimens. If one test specimen shows evidence of improper sampling, molding, or testing, it shall be discarded and the remaining specimens tested. The strengths of the remaining two specimens shall be averaged, and the result shall then be considered the compressive strength of the concrete. If more than one specimen shows such defects, the test is not valid and the remaining specimen shall be discarded.
5. If test results are invalid due to specimen defects, or the in-place concrete that is in question was not sampled, the in-place concrete may be sampled by coring in accordance with ASTM C 42. For core tests, at least three representative cores shall be taken from each area of the concrete in question. If one or more of the cores shows signs of being damaged before testing, it shall be replaced by a new one.

## **16. Specific Site Requirements**