

NATURAL RESOURCES CONSERVATION SERVICE
CONSERVATION PRACTICE STANDARD

GRADE STABILIZATION STRUCTURE

(NO.)
CODE 410

Definition

A structure used to control the grade and head cutting in natural or artificial channels.

Purpose

To stabilize the grade and control erosion in natural or artificial channels, to prevent the formation or advance of gullies, and to enhance environmental quality and reduce pollution hazards.

Conditions where practice applies

In areas where the concentration and flow velocity of water require structures to stabilize the grade in channels or to control gully erosion. Special attention shall be given to maintaining or improving habitat for fish and wildlife where applicable.

Design criteria

The structure must be designed for stability after installation. The crest of the inlet must be set at an elevation that stabilizes upstream head cutting.

Embankment dams. Class (a) dams that have product of storage times the effective height of the dam of 3,000 or more, those more than 35 ft in effective height, and all class (b) and class (c) dams shall meet or exceed the requirements specified in

Technical Release No. 60 (TR-60).

Class (a) dams that have a product of storage times the effective height of the dam of less than 3,000 and an effective height of 35 ft or less shall meet or exceed the requirements specified for ponds (378).

The effective height of the dam is the difference in elevation, in feet, between the emergency spillway crest and the lowest point in the cross section along the centerline of the dam. If there is no emergency spillway, the top of the dam is the upper limit.

Pond size dams. If mechanical spillways are required, the minimum capacity of the principal spillway shall be that required to pass the peak flow expected from a 24-hour duration design storm of the frequency shown in table OK-1, less any reduction because of detention storage. The emergency spillway condition shall be determined by requirements specified for ponds.

If the effective height of the dam is less than 20 ft and the emergency spillway has a stable grade throughout its length with no overfalls and has good vegetation along its reentry into the downstream channel, the principal spillway capacity may be reduced but can be no less than 80 percent of the 2-year frequency, 24-hour duration storm.

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If criteria values exceed those shown in table OK-1 or the storage capacity is more than 50 acre-ft, the 10-year frequency, 24-hour duration storm must be used as the minimum design storm.

Table OK-1

Design criteria for establishing minimum capacity of the principal spillway for dams with storage capacity of less than 50 acre-feet at emergency spillway elevation

Maximum Drainage Area For Indicated Rainfall*		Eff. Height of Dam ft	Frequency of Minimum Design - 24 Hour Duration Storm		
3-5 in.	5+ in.		Emer. Spillway Condition		Very Poor
acres			Fair	Poor	
100	50	35 or less	2	2	5
200	100	20 or less	2	2	5
200	100	20-35	5	5	10
400	200	20 or less	2	5	10
800	400	20 or less	2	10	10

*In a 5-year frequency, 24-hour duration storm.

Except as noted below, the design of embankments, principal and emergency spillways, and foundation and embankment drains shall be according to NRCS standards for ponds (378).

Grade stabilization structures with a settled fill height of less than 15 ft and 10-year frequency, 24-hour storm runoff less than 10 acre-ft, shall be designed to control the 10-year frequency storm without overtopping. The mechanical spillway, regardless of size, may be considered in design and an emergency spillway is not required if the combination of storage and mechanical spillway discharge will handle the design storm. The embankment can be designed to meet the requirements for water and sediment control basins (638)

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rather than the requirements for ponds (378).

Full-flow open structures. Drop, chute, and box inlet drop spillways shall be designed according to the principles set forth in the Engineering Field Handbook Part 650, the National Engineering Handbook, and other applicable NRCS publications and reports. The minimum capacity shall be that required to pass the peak flow expected from a design storm of the frequency and duration shown in table 2, less any reduction because of detention storage.

If site conditions exceed those shown in table 2, the minimum design 24-hour storm frequency is 25 years for the principal spillway and 100 years for the total capacity. Structures must not create unstable conditions upstream or downstream. Provisions must be made to insure reentry of bypassed storm flows.

Table 2. - Design criteria for establishing minimum capacity of full-flow open structures.

Max drainage area for indicated rainfall*			Frequency of min. design, 24-hour duration storm		
0-3 in	3-5 in	5+ in	Vertical drop	Principal spillway capacity	Total capacity
acres			ft	yr	yr
1,200	450	250	5 or less	5	10
2,200	900	500	10 or less	10	25

*In a 5-year frequency, 24-hour duration storm.

Toe wall drop structures can be used if the vertical slope is 4 ft or less, flows are intermittent, downstream grades are stable, and tail water depth at design flow is equal

to or greater than one-third of the height of the overfall.

The ratio of the capacity of drop boxes to road culverts shall be as required by the responsible road authority or as specified in table 2 or 3, as applicable, less any reduction because of detention storage, whichever is greater. The drop box capacity (attached to a new or existing culvert) must equal or exceed the culvert capacity at design flow.

Island-type structures. If the mechanical spillway is designed as an island-type structure, its minimum capacity shall equal the capacity of the downstream channel. For channels with very small drainage areas, the mechanical spillway should carry at least the 2-year, 24-hour storm or the design drainage curve runoff. The minimum emergency spillway capacity shall be that required to pass the peak flow expected from a design storm of the frequency and duration shown in table 2 for total capacity without overtopping the headwall extensions of the mechanical spillway. Provision must be made for safe reentry of bypassed flow as necessary.

Table 3. - Design criteria for establishing minimum capacity of side-inlet, open-weir, or pipe-drop-drainage structures.

Max drainage area for indicated rainfall*			Frequency of min. design, 24-hour duration storm		
			Vertical drop	Receiving channel	Total capacity
0-3 in	3-5 in	5+ in		depth	
acres			ft	ft	yr
1,200	450	250	0-5	0-10	---
1,200	450	250	5-10	10-20	10
2,200	900	500	0-10	0-20	25

*In a 5-year frequency, 24-hour duration storm.

Side-inlet drainage structures. The design criteria for minimum capacity of open-weir or pipe structures used to lower surface water from field elevations or lateral channels into deeper open channels are shown in table 3. The minimum principal spillway capacity shall equal the design drainage curve runoff for all conditions. If site condition values exceed those shown in table 3, the 50-year frequency storm shall be used for minimum design of total capacity.

Landscape resources. In highly visible public areas and those associated with recreation, careful considerations should be given to landscape resources. Landforms, structural materials, water elements, and plant materials should visually and functionally complement their surroundings. Excavated material and cut slopes should be shaped to blend with the natural topography. Shorelines can be shaped and islands created to add visual interest and valuable wildlife habitat. Exposed concrete surfaces may be formed to add texture or finished to reduce reflection and to alter color contrast. Site selection can be used to reduce adverse impacts or create desirable focal points.

General criteria. Earth embankment and emergency spillways of structures for which criteria are not provided under the standard for ponds (378) or in TR-60 must be stable for all anticipated conditions. If earth spillways are used, they must be designed to handle the total capacity flow indicated in tables 2 or 3 without overtopping the dam. The foundation preparation, compaction, top width, and side slopes must ensure a stable dam for anticipated flow conditions. Discharge from the structure shall be sufficient that no crop damage results from flow detention.

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Necessary sediment storage capacity must equal the expected life of the structure, unless a provision is made for periodic cleanout. The expected life of the structure shall be 20 years.

The earth embankment pond structures are potentially hazardous and precautions must be taken to prevent serious injury or loss of life. Protective guardrails, warning signs, fences, or lifesaving equipment shall be added as needed.

If the area is used for livestock, the structures, earthfill, vegetated spillways, and other areas should be fenced as necessary to protect the structure. Near urban areas, fencing may be necessary to control access and exclude traffic that may damage the structure or to prevent serious injury or death to trespassers.

Protection. The exposed surfaces of the embankment, earth spillway, borrow area, and other areas disturbed during construction shall be seeded or sodded as necessary to prevent erosion in accordance with practice standard Critical Area Planting (342). If climatic conditions preclude the use of vegetation, nonvegetative coverings such as gravel or other mulches may be used.

Plans and specifications

Plans and specifications for installing grade stabilization structures shall be in keeping with this standard and shall describe the requirements for applying the practice to achieve its intended purpose.

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NATURAL RESOURCES CONSERVATION SERVICE
CONSERVATION PRACTICE GENERAL SPECIFICATIONS

GRADE STABILIZATION STRUCTURE
(NO.)
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**GRADE STABILIZATION STRUCTURE
SPECIFICATIONS**

Specifications for grade stabilization structures within the scope of the standard for water and sediment control basins (638) shall be meet those specifications.

Specifications for grade stabilization structures within the scope of the standard for ponds (378) shall, as a minimum, be commensurate with those for ponds. Grade stabilization structures within the scope of TR-60 shall be constructed according to the guide specifications in the National Engineering Handbook, Section 20.

Concrete

Concrete placement tolerances shall be as follows:

1. Concrete slab thickness shall not be more than 1/2 inch less than the designed thickness measured perpendicular to the surface.
2. Formless concrete chute inlet crest shall be within ± 0.1 foot of the design elevation.
3. Outlet floor of the formless concrete chute shall not be more than 0.1 foot above the design elevation.
4. All other elevations and dimensions for formless concrete chutes or other concrete slabs shall equal or exceed design requirements.

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