

**NATURAL RESOURCES CONSERVATION SERVICE
CONSERVATION PRACTICE STANDARD**

WASTE STORAGE FACILITY

(No.)

CODE 313

DEFINITION

A waste storage impoundment made by constructing an embankment and/or excavating a pit or dugout or by fabricating a structure.

PURPOSE

To temporarily store wastes such as manure, wastewater, and contaminated runoff as a storage function component of an agricultural waste management system.

CONDITIONS WHERE PRACTICE APPLIES

- Where the storage facility is a component of a planned agricultural waste management system.
- Where temporary storage is needed for organic wastes generated by agricultural production or processing.
- Where the storage facility can be constructed, operated, and maintained without polluting air or water resources.
- Where site conditions are suitable for construction of the facility.
- To facilities utilizing embankments with an effective height of 35 feet or less where damage resulting from failure would be limited of farm buildings, agricultural land, or township and country roads.
- To fabricated structures including tanks, stacking facilities, and pond appurtenances.

CRITERIA

General Criteria Applicable to All Waste Storage Facilities

Laws and regulations. Waste storage facilities must be planned, designed, and constructed to meet all federal, state, and local laws and regulations.

Before construction begins, all required plans and specifications for facilities shall be submitted to the Kansas Department of Health and Environment (KDHE) for review and approval. Construction permits may also be required from the Kansas Department of Agriculture, Division of Water Resources (DWR), for embankment ponds or structures within the 100-year floodplain. The owner or operator of the facility shall be responsible for securing all required permits and approvals from the agency or agencies concerned.

Location. To minimize the potential for contamination of streams, waste storage facilities should be located outside of floodplains. However, if site restrictions require location within a floodplain, they shall be protected from inundation or damage from a 25-year flood event (or larger) if required by laws, rules, and regulations. Waste storage facilities shall be located so that the potential impacts from breach of embankment, accidental release, and liner failure are minimized and separation distances are such that prevailing winds and landscape elements (such as building arrangement, landforms, and vegetation) minimize odors and protect aesthetic values.

The location of waste storage facilities must also conform to the separation distance requirements relating to habitable structures, water resources, and property lines.

Storage period. The storage period is the maximum length of time anticipated between emptying events. The minimum storage period shall be based on the timing required for environmentally safe waste utilization considering the climate, crops, soil, equipment, and local, state, and federal regulations. However, it shall not be less than 120 days and is recommended to be the most critical 120-day period (for example, winter or high inflow months).

Design storage volume. The design storage volume equal to the required storage volume shall consist of the total of the following as appropriate:

- Manure, wastewater, and other wastes accumulated during the storage period.
- Normal precipitation less evaporation on the surface area (at the design storage volume level) of the facility during the storage period.
- Normal runoff from the drainage area of the facility during the storage period.
- 25-year, 24-hour precipitation on the surface (at the required design storage volume level) of the facility.
- 25-year, 24-hour runoff from the drainage area of the facility.
- Residual solids after liquids have been removed—a minimum of 6 inches shall be provided for tanks.
- Additional storage as may be required to meet management goals or regulatory requirements.

Inlets. Inlets shall be of any permanent type that is designed to resist corrosion, plugging, freeze damage, and ultraviolet ray deterioration while incorporating erosion protection as necessary.

Conduits shall conform to the criteria indicated in [Conservation Practice Standard \(CPS\) 634, Waste Transfer](#).

Emptying component. Some type of component shall be provided for emptying

storage facilities. This component may be a gate, pipe, dock, wet well, pumping platform, retaining wall, or ramp. Features to protect against erosion, tampering, and accidental release shall be incorporated as necessary.

The capacity of pumps used as components of the emptying system shall meet the operational requirements of the producer.

Accumulated solids removal. Provision shall be made for periodic removal of accumulated solids to preserve storage capacity. The anticipated method for doing this must be considered in planning, particularly in determining the configuration of ponds and type of seal (if any).

Safety. Design shall include appropriate safety features to minimize the hazards of the facility. Ramps used to empty liquids shall have a slope of 4 horizontal to 1 vertical (4:1) or flatter. Those used to empty slurry, semi-solid, or solid waste shall have a slope of 10:1 or flatter unless special traction surfaces are provided.

Warning signs, fences, ladders, ropes, bars, rails, and other devices shall be provided (as appropriate) to ensure the safety of humans and livestock. Ventilation and warning signs must be provided for covered waste holding structures (as necessary) to prevent explosion, poisoning, or asphyxiation. Pipelines shall be provided with a water-sealed trap and vent or similar device if there is a potential, based on design configuration, for gases to enter buildings or other confined spaces.

Ponds and uncovered fabricated structures for liquid or slurry waste with walls less than 5 feet above ground surface shall be fenced, and warning signs shall be posted to prevent children and others from using them for other than their intended purpose.

Erosion protection. Embankments and disturbed areas surrounding the facility shall be treated to control erosion. Refer to [CPS 342, Critical Area Planting](#), for additional criteria.

Additional Criteria for Waste Storage Ponds

Soil and foundation. The pond shall be located in soils with an acceptable permeability that meets all applicable regulation, or the pond shall be lined. Information and guidance on controlling seepage from waste impoundments

can be found in [National Engineering Handbook Part 651 \(NEH 651\), Agricultural Waste Management Field Handbook, Appendix 10D.](#)

The pond shall have a bottom elevation that is a minimum of 10 feet above the seasonal high water table unless features of special design are incorporated that address buoyant forces, pond seepage rate, and non-encroachment of the water table by contaminants. The seasonal high water table may be lowered by use of perimeter drains, if feasible, to meet this requirement.

Geotechnical investigations. All pond sites shall be investigated to determine water table elevations and to identify soil types. Site investigations will consist of borings or equivalent excavations with at least 1 investigation to a minimum depth of 10 feet below the lowest elevation in a waste storage pond or where impenetrable bedrock is encountered—whichever is less. Other investigations are recommended to be a minimum of 2 feet below the lowest elevation of the waste storage pond. The depth to the seasonal high water table (if encountered) shall be documented.

The extent of the investigation shall be commensurate with the complexity of the site geology and the potential hazard posed by the pond. It is recommended that at least 1 soil investigation be performed for each acre occupied by a storage pond, a minimum of 1 investigation is required—regardless of size. The area occupied by the pond is the surface area measured at the design top elevation. Additional information on geologic investigations can be found in [Chapter 7 in NEH Part 651.](#)

Information and guidance on soil profile descriptions can be found in [Chapter 3 in the Soil Survey Manual](#) (U.S. Department of Agriculture [USDA] Handbook 18, October 1993) or in the [Field Book for Describing and Sampling Soils, Version 3.0](#) (USDA, NRCS, National Soil Survey Center, 2012).

Liners. Liners, if required, shall be used to seal the bottoms and sides of ponds (at least to the design storage volume elevation) and shall be accomplished by 1 of the following methods or materials:

- Compacting of native clay soils (on-site or imported).

- Adding bentonite to the soil.
- Adding a soil dispersant to the soil.
- Covering the soil with commercial flexible membranes, geosynthetic clay liner, or concrete.

Liners shall meet or exceed the criteria in [Conservation Practice Standard \(CPS\) 521A, Pond Sealing or Lining, Flexible Membrane](#); [CPS 521B, Pond Sealing or Lining, Soil Dispersant Treatment](#); [CPS 521C, Pond Sealing or Lining, Bentonite Treatment](#); or [CPS 521D, Pond Sealing or Lining, Compacted Clay Treatment](#), as appropriate.

Maximum operating level. The maximum operating level for waste storage ponds shall be the pond level that provides for the required volume less the volume contribution of precipitation and runoff from the 25-year, 24-hour storm event plus the volume allowance for residual solids after liquids have been removed. A permanent marker or recorder shall be installed to indicate this maximum operating level and when drawdown should begin. The marker or recorder shall be referenced and explained in the operation and maintenance (O&M) plan or the comprehensive nutrient management plan (CNMP).

Outlet. No outlet shall automatically release storage from the required design volume. Manually operated outlets shall be of a permanent type that is designed to resist corrosion and plugging. Gravity discharge pipes used for emptying a waste storage pond shall have a minimum of 2 gates or valves—1 of which shall be manually operated. Conduits placed through embankment ponds shall conform to the criteria indicated in [CPS 378, Pond](#), for seepage protection.

Embankments. The minimum elevation of the top of the settled embankment shall be 2 feet (for freeboard) above the design storage volume elevation of the waste storage pond. This height shall be increased by the amount needed to ensure that the top elevation will be maintained after settlement. This increase shall not be less than 5 percent. The minimum top widths are shown in Table 1.

Table 1—Minimum top widths

Total Embankment Height (feet)	Top Width (feet)
20 or less	10
20.1 - 25	12
25.1 - 30	14
30.1 - 35	15

Side slopes of the settled embankment shall not be steeper than is shown in Table 2. All slopes must be designed to be stable—even if flatter side slopes are required.

Table 2—Minimum side slopes

Total Embankment Height (feet)	Inside Slope	Outside Slope
Less than 12	3:1	2:1
Greater than or equal to 12	3:1	2½:1

Cutoff trenches, wave erosion control measures, and auxiliary spillways shall be provided as needed to ensure safe and proper performance of embankment structures. Refer to [CPS 378](#) for criteria applicable to these measures.

Excavations. Side slopes of excavated ponds shall be stable against sloughing. Side slopes of 3:1 or flatter are preferred and should be used unless site conditions or machinery limitations require steeper slopes. In no case shall the side slopes be steeper than 2:1.

Additional Criteria for Fabricated Structures

Geotechnical investigations. The extent of the investigation shall be commensurate with the complexity of the site geology, soils, and the potential hazard posed by the structure. The investigation shall be adequate to evaluate foundation stability, settlement potential, and groundwater protection.

Foundation. The foundations of fabricated waste storage structures shall be proportioned to safely support all superimposed loads without excessive movement or settlement.

Where a nonuniform foundation cannot be avoided or applied loads may create highly variable foundation loads, settlement should be

calculated from site-specific soil test data. Index tests of site soils may allow correlation with similar soils for which test data is available. If no test data is available, presumptive bearing strength values for assessing actual bearing pressures may be obtained from Table 3 or another nationally recognized building code. In using presumptive bearing values, adequate detailing and articulation shall be provided to avoid distressing movements in the structure.

Foundations consisting of bedrock with joints, fractures, or solution channels shall be treated or a separation distance shall be provided consisting of a minimum of 1 foot of impermeable soil between the floor slab and the bedrock or an alternative that will achieve equal protection.

Table 3—Presumptive allowable bearing stress values¹

Foundation Description	Allowable Stress
Crystalline bedrock	12,000 psf
Sedimentary rock	6000 psf
Sandy gravel or gravel	5000 psf
Sand, silty sand, clayey sand, silty gravel, or clayey gravel	3000 psf
Clay, sandy clay, silty clay, or clayey silt	2000 psf

¹ Basic Building Code, 12th Edition, 1993, Building Officials and Code Administrators, Inc. (BOCA)

Freeboard. A minimum of 0.5 foot of freeboard shall be provided in addition to the design storage volume.

Liquid tightness. Applications such as tanks that require liquid tightness shall be designed and constructed in accordance with standard engineering and industry practices appropriate for the construction materials used to achieve this objective. Applicable products include (but are not limited to) waterstops, geomembranes, composite drainage and lining materials, and chemical sealants. Materials and installation shall conform to the manufacturer's recommendations.

Structural loadings. Waste storage structures shall be designed to withstand all anticipated loads including internal and external loads, hydrostatic uplift pressure, concentrated surface and impact loads, water pressure due to seasonal high water table, and frost or ice pressure and load combinations in compliance with this standard and applicable local building codes.

The lateral earth pressures should be calculated from soil strength values determined from the results of appropriate soil tests. Lateral earth pressures can be calculated using the procedures in [Technical Release 74](#). If soil strength tests are not available, the presumptive lateral earth pressure values indicated in Table 4 shall be used.

Table 4—Lateral earth pressure values¹

Soil		Equivalent Fluid Pressure (lb/ft ² /ft of depth)			
		Above Seasonal High Water Table ²		Below Seasonal High Water Table ³	
Description ⁴	Unified Classification ⁴	Free-Standing Walls	Frame Tanks	Free-Standing Walls	Frame Tanks
Clean gravel, sand, or sand-gravel mixtures (maximum 5% fines) ⁵	GP, GW, SP, SW	30	50	80	90
Gravel, sand, silt, and clay mixtures (less than 50% fines) Coarse sands with silt and/or clay (less than 50% fines)	All gravel sand dual symbol classifications and GM, GC, SC, SM, SC-SM	35	60	80	100
Low-plasticity silts and clays with some sand and/or gravel (50% or more fines) Fine sands with silt and/or clay (less than 50% fines)	CL, ML, CL-ML SC, SM, SC-SM	45	75	90	105
Low- to medium-plasticity silts and clays with little sand and/or gravel (50% or more fines)	CL, ML, CL-ML	65	85	95	110
High plasticity silts and clays (liquid limit more than 50) ⁶	CH, MH	-	-	-	-

¹ For lightly compacted soils (85% to 90% maximum standard density)—includes compaction by use of typical farm equipment

² Also below seasonal high water table if adequate drainage is provided

³ Includes hydrostatic pressure

⁴ All definitions and procedures in accordance with American Society for Testing and Materials (ASTM) D2488 and D653

⁵ Generally, only washed materials are in this category

⁶ Not recommended—requires special design if used

Lateral earth pressures based upon equivalent fluid assumptions shall be assigned according to the following conditions:

- Rigid frame or restrained wall—use the values shown in Table 4 under the column “Frame Tanks” that give pressures comparable to the at-rest condition.
- Flexible or yielding wall—use the values shown in Table 4 under the column “Free-Standing Walls” that give pressures comparable to the active condition. Walls in this category are designed on the basis of gravity for stability or are designed as a cantilever having a base wall thickness to height of backfill ratio not more than 0.085.

Internal lateral pressure used for design shall be 65 pounds per square foot (psf) where the stored waste is not protected from precipitation. A value of 60 psf may be used where the stored waste is protected from precipitation and will not become saturated. Lesser values may be used if supported by measurement of actual pressures of the waste to be stored. If heavy equipment will be operated near the wall, an additional 2 feet of soil surcharge shall be considered in the wall analysis.

Tank covers shall be designed to withstand both dead and live loads. The live load values for covers contained in American Society of Agricultural Engineers (ASAE) EP378.4, Floor and Suspended Loads on Agricultural Structures Due to Use, and in ASAE EP393.3, Manure Storages, shall be the minimum used. The actual axle load for tank wagons having more than a 2,000-gallon capacity shall be used.

If the facility is to have a roof, snow and wind loads shall be as specified in American Society of Civil Engineers (ASCE) 7, Minimum Design Loads for Buildings and Other Structures. If the facility is to serve as part of a foundation or support for a building, the total load shall be considered in the structural design.

Structural design. The structural design shall consider all items that will influence the performance of the structure—including loading assumptions, material properties, and construction quality. Design assumptions and limitations shall be documented in the design file

and may be listed on the drawings. All pertinent construction requirements shall be clearly indicated in the construction drawings and specifications.

Tanks may be designed with or without covers. Covers, beams, or braces that are integral to structural performance must be indicated on the construction drawings. The openings in covered tanks shall be designed to accommodate equipment for loading, agitating, and emptying. These openings shall be equipped with grills or secure covers for safety and for odor and vector control.

All structures shall be underlain by free-draining material or shall have a footing located below the anticipated frost depth. Fabricated structures shall be designed according to the criteria in the following references as appropriate:

- Steel: “Manual of Steel Construction,” American Institute of Steel Construction.
- Timber: “National Design Specifications for Wood Construction,” American Wood Council.
- Concrete: American Concrete Institute (ACI) 318, “Building Code Requirements for Reinforced Concrete.”
- Masonry: ACI 530, “Building Code Requirements for Masonry Structures.”

Slabs-on-grade. Slab design shall consider the required performance and the critical applied loads along with both the subgrade material and material resistance of the concrete slab. Where applied point loads are minimal and liquid-tightness is not required (such as barnyard and feedlot slabs subject only to precipitation) and the subgrade is uniform and dense, the minimum slab thickness shall be 4 inches with a maximum joint spacing of 10 feet. Joint spacing can be increased if steel reinforcing is added based on subgrade drag theory.

For applications where liquid-tightness is required (such as floor slabs of storage tanks), the minimum thickness for uniform foundations shall be 5 inches and shall contain distributed reinforcing steel. The required area of such reinforcing steel shall be based on subgrade drag theory as discussed in industry guidelines such as ACI 360, “Design of Slabs-on-Grade.”

When heavy equipment loads are to be resisted or where a nonuniform foundation cannot be avoided, an appropriate design procedure incorporating a subgrade resistance parameter such as ACI 360 shall be used.

Additional Criteria for Temporary Waste Field Storage Facilities

Short-term stockpiling of dry manure/litter near the application area may be acceptable when direct application is temporarily not feasible. This manure/litter will be solid animal manures (more than 25% solids) and will be covered.

A nutrient management plan (NMP) on the application fields is required before the stockpiling component can be used. The plan map shall show the location of all storage areas, access roads to these areas, slopes, surfaces to be graded, necessary cuts and fills, and location of sites subject to pollution such as wells, springs, streams, and floodplains. Auxiliary practices such as access roads, diversions, waterways, subsurface drains, and vegetation shall be used and shown on the plan maps as required.

Location. Waste field storage shall be located as follows:

- No farther than 150 feet from the top of a slope unless a diversion is installed.
- At least 1 foot vertically above the floodplain of the 25-year, 24-hour storm.
- Where year-round access to the manure storage will be practical during periods of wet weather.
- At least 150 feet from wells, springs, streams, and ponds or 300 feet from a well when the well is located downgradient from the storage area.
- At least 300 feet from neighboring residences or public areas.
- Near natural windbreaks (where possible) to protect the covering from blowing winds.
- Where the seasonal high water table will be no closer than 3 feet below the bottom of the stored manure—unless a concrete pad or synthetic liner is used.
- Within limits as required by state laws and regulations.

Covering. The covering can be temporary or permanent. A permanent cover should meet the requirements of [CPS 367, Roofs and Covers](#). A temporary cover for field-stored manure/litter shall be opaque plastic sheeting that has a minimum thickness of 6 mils. The sheeting must be placed over the pile with care to prevent tearing. Weighted objects which will not damage the plastic shall be placed over the sheeting to anchor it and prevent tearing during high winds. The covering shall be trenched, diked, or otherwise fastened at the perimeter of the pile to prevent any contact by rainfall runoff with the manure/litter and to prevent uncovering of the edge of the pile. One option is a 12-inch deep trench that is constructed around the waste and the edges of the sheeting buried in and through the trench.

Size. Waste field storage shall be designed to store up to 50% of the total volume to be applied as required for proper nutrient management as identified in the NMP. Long-term storage for periods of time greater than 180 days shall be accommodated by using a permanent structure that meets the requirements of this standard.

The size of the pad on which the solid manure/litter will be stored shall be determined on the basis of manure volume produced and the anticipated height of the stack (maximum height 7 feet plus any wall height). Sufficient horizontal freeboard shall be allowed around the edges of the stack to properly anchor the covering. The pad shall have positive drainage from the stack in all directions or shall be protected from runoff by diversions.

Soils and foundation. Field storage for the manure/litter shall be placed on a raised pad to facilitate drainage. This pad can be constructed of 1 of the following:

- 5-inch thick concrete.
- Synthetic liner that meets the requirements of [CPS 521A, Pond Sealing or Lining, Flexible Membrane](#).
- A 1-foot layer of compacted clayey material that meets the requirements of [CPS 521D, Pond Sealing or Lining, Compacted Clay Treatment](#).

If a synthetic liner is used, the pad should be overexcavated 1 foot, and all sharp stones and other sharp material shall be removed to prevent

puncturing the liner. The liner should then be covered with 1 foot of the best fine-grained soil that is locally available.

Soil pads shall be installed under optimum moisture conditions and compacted in 6- to 8-inch lifts. The pad shall be essentially level with only enough gradient away from the center of the pad to allow drainage of water. All excavated side slopes shall not be steeper than 3:1.

Vegetation. All disturbed areas beyond the edges of the stored manure/litter shall be seeded to an approved dense, permanent, vegetative cover as shown in the plan. If the stockpile is not covered, then the vegetated area downgradient from the site shall be a minimum of 50 feet wide.

CONSIDERATIONS

Waste storage facilities should be located as close to the source of waste and polluted runoff as practicable.

Nonpolluted runoff should be excluded from the structure to the fullest extent possible except where its storage is advantageous to the operation of the agricultural waste management system.

Solid/liquid separation of runoff or wastewater entering pond facilities should be considered to minimize the frequency of accumulated solids removal and to facilitate pumping and application of the stored waste.

Due consideration should be given to environmental concerns, economics, the overall waste management system plan, and safety and health factors.

Consider having a sample of the soil to be used for the soil liner sent to a soil mechanics laboratory for tests to determine the density, moisture, and amendment requirements that are needed to meet the required permeability.

Considerations for Minimizing the Potential for and Impacts of Sudden Breach of Embankment or Accidental Release from the Required Volume

Features, safeguards, or management measures to minimize the risk of failure or accidental release or to minimize or mitigate impact of this type of failure should be considered when any of the categories listed in Table 5 might be significantly affected.

Table 5—Potential impact categories from breach of embankment or accidental release

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| <ol style="list-style-type: none"> 1. Surface water bodies—perennial streams, lakes, wetlands, and estuaries. 2. Critical habitat for threatened and endangered species. 3. Riparian areas. 4. Farmstead or other areas of habitation. 5. Off-farm property. 6. Historical and/or archaeological sites or structures that meet the eligibility criteria for listing in the National Register of Historical Places. |
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The following should be considered either singly or in combination to minimize the potential of or the consequences of sudden breach of embankment when 1 or more of the potential impact categories listed in Table 5 may be significantly affected:

- An auxiliary spillway.
- Additional freeboard.
- Storage for wet year rather than normal year precipitation.
- Reinforced embankment such as additional top width or side slopes that are flattened or armored downstream.
- Secondary containment.

The following options should be considered to minimize the potential for accidental release from the required volume through gravity outlets when 1 or more of the potential impact categories listed in Table 5 may be significantly affected:

- Outlet gate locks or locked gate housing.

- Secondary containment.
- Alarm system.
- Another means of emptying the required volume.

Considerations for Minimizing the Potential of Waste Storage Pond Liner Failure

Sites with categories listed in Table 6 should be avoided unless no reasonable alternative exists. Under those circumstances, consideration should be given to providing an additional measure of safety from pond seepage when any of the potential impact categories listed in Table 6 may be significantly affected.

Table 6–Potential impact categories for liner failure

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| <ol style="list-style-type: none"> 1. Any underlying aquifer is at a shallow depth and not confined. 2. The vadose zone is rock. 3. The aquifer is a domestic water supply or ecologically vital water supply. 4. The site is located in an area of solutionized bedrock such as limestone or gypsum. |
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Should any of the potential impact categories listed in Table 6 be affected, consideration should be given to the following:

- A clay liner designed in accordance with procedures in [Appendix 10D in NEH 651](#) with a thickness and coefficient of permeability so that specific discharge is less than 1×10^{-6} centimeters per second.
- A flexible membrane liner over a clay liner.
- A geosynthetic clay liner (GCL) flexible membrane liner.
- A concrete liner designed in accordance with slabs-on-grade criteria for fabricated structures requiring water-tightness.

Considerations for Improving Air Quality

To reduce emissions of greenhouse gases, ammonia, volatile organic compounds, and odor, other conservation practices such as [366, Anaerobic Digester](#); [367, Roofs and Covers](#); and [317, Composting Facilities](#), can be added to the waste management system.

Adjusting pH below 7 may reduce ammonia emissions from the waste storage facility but may increase odor when waste is surface-applied (see [CPS 633, Waste Recycling](#)).

Some fabric and organic covers have been shown to be effective in reducing odors.

PLANS AND SPECIFICATIONS

Plans and specifications shall be prepared in accordance with the criteria of this standard and shall describe the requirements for applying the practice to achieve its intended use.

OPERATION AND MAINTENANCE

An O&M plan shall be developed that is consistent with the purposes of the practice, its intended life, safety requirements, and the criteria for its design.

The plan shall contain the operational requirements for emptying the storage facility. This shall include the requirement that waste shall be removed from storage and used at locations, times, rates, and volume in accordance with the overall waste management system plan.

In addition, for ponds, the plan shall include an explanation of the permanent marker or recorder installed to indicate the maximum operating level.

The plan shall include a strategy for removal and disposition of waste with the least environmental damage during the normal storage period to the extent necessary to ensure the safe operation of the pond. This strategy shall also include the removal of runoff from large storm events that may cause the pond to fill to capacity prematurely with subsequent design inflow and usual precipitation prior to the end of the normal storage period.

Development of an emergency action plan should be considered for waste storage facilities where there is a potential for significant impact from breach or accidental release. The plan shall include site-specific provisions for emergency actions that will minimize these impacts.