Natural Resources Conservation Service

CONSERVATION PRACTICE STANDARD

POND

Code 378

(No.)

DEFINITION

A pond is a water impoundment made by constructing an embankment, by excavating a dugout, or by a combination of both.

In this standard, NRCS defines ponds constructed by the first method as embankment ponds, and those constructed by the second method as excavated ponds. Ponds constructed by both the excavation and the embankment methods are classified as embankment ponds if the depth of water impounded against the embankment at the auxiliary spillway elevation is 3 feet or more above the lowest original ground along the centerline of the embankment.

PURPOSE

A pond stores water for livestock, fish and wildlife, recreation, fire control, erosion control, flow detention, and other uses such as improving water quality.

CONDITIONS WHERE PRACTICE APPLIES

This practice applies to all excavated ponds. It also applies to embankment ponds that meet all of the criteria for low-hazard dams as listed below:

- The failure of the dam will not result in loss of life, damage to homes, commercial or industrial buildings, main highways, or railroads, or in interruption of the use or service of public utilities.
- The product of the storage times the effective height of the dam is less than 3,000 acre-feet[2]. Storage is the capacity of the reservoir in acre-feet below the elevation of the crest of the lowest auxiliary spillway or the elevation of the top of the dam if there is no open channel auxiliary spillway. The effective height of the dam is the difference in elevation, in feet, between the lowest open channel auxiliary spillway crest and the lowest point in the original cross section taken on the centerline of the dam. If there is no open channel auxiliary spillway, use the lowest point on the top of the dam instead of the lowest open channel auxiliary spillway crest.
- The effective height of the dam is 35 feet or less.

CRITERIA

General Criteria Applicable to All Purposes

Design a minimum sediment storage capacity equal to the design life of the structure, or provide for periodic cleanout. Protect the drainage area above the pond to prevent sedimentation from adversely affecting the design life.

Drainage area. The drainage area shall be large enough so that surface runoff and groundwater flow will maintain an adequate supply of water in the pond. A drainage area to surface area ratio of 10:1 will provide a reliable water supply to replenish evaporation and seepage losses. The quality shall be suitable for the water's intended use.

Design measures necessary to prevent serious injury or loss of life in accordance with requirements of NRCS National Engineering Manual (NEM), Part 503, Safety.

Seed or sod the exposed surfaces of earthen embankments, earth spillways, borrow areas, and other areas disturbed during construction in accordance with the criteria in NRCS Conservation Practice Standard (CPS) Code 342, Critical Area Planting

Cultural resources. Evaluate the existence of cultural resources in the project area and any project impacts on such resources. Provide conservation and stabilization of archeological, historic, structural, and traditional cultural properties when appropriate.

Site conditions. Select or modify the site to allow runoff from the design storm to safety pass through (1) a natural or constructed auxiliary spillway, (2) a combination of a principal spillway and an auxiliary spillway, or (3) a principal spillway.

Select a site that has an adequate supply of water for the intended purpose via surface runoff, groundwater, or a supplemental water source. Water quality must be suitable for its intended use.

Reservoir. Provide adequate storage volume to meet user demands for all intended purposes. Account for sedimentation, season of use, evaporation loss, and seepage loss when computing the storage volume.

The topography and soils of the site shall permit storage of water at a depth and volume that ensure a dependable supply, considering beneficial use, sedimentation, season of use, and evaporation and seepage losses. If surface runoff is the primary source of water for a pond, the soils shall be impervious enough to prevent excessive seepage losses or shall be of a type that sealing is practicable. Ponds for stock water or scale fish production shall provide a minimum water depth of 6 feet over 20 percent of the water surface area at spillway level for embankment ponds and 25 percent of the surface area for pit or dugout ponds. For catfish ponds, a depth of 4 to 5 feet is satisfactory. The majority of the water area should have a depth greater than 2.0 feet. Deepening of the pond edge may be desirable to control growth of undesirable vegetation, to improve fish habitat, or to supply embankment borrow material.

Side slope of the deepened area should be between 3:1 and 1.5:1 with the finished bank 3 inches or more above crest of principal spillway. Woody vegetation and scattered trees in the pool area may be left standing in small tributary inlets and other selected locations to enhance the fishery habitat but should not exceed 10 percent of the pool area.

Criteria Applicable to Embankment Ponds.

Geological investigations. Use pits, trenches, borings, and reviews of existing data or other suitable means of investigation to characterize materials within the embankment foundation, auxiliary spillway, and borrow areas. Classify soil materials using the Unified Soil Classification System (ASTM D2487).

Foundation cutoff. Design a cutoff of relatively impervious material under the dam and up the abutments as required for preventing seepage. Locate the cutoff at, or upstream from, the centerline of the dam. Extend the cutoff deep enough to intercept flow and connect with a relatively impervious layer. Combine seepage control with the cutoff as needed. Use a cutoff bottom width adequate to accommodate the equipment used for excavation, backfill, and compaction operations. Design cutoff side slopes no steeper than one horizontal to one vertical.

Seepage control. Include seepage control if (1) foundation cutoff does not intercept pervious layers, (2) seepage could create undesired wet areas, (3) embankment stability requires seepage control, or (4) special problems require drainage for a stable dam. Control seepage by (1) foundation, abutment, or embankment filters and drains; (2) reservoir blanketing; or (3) a combination of these measures.

Top width. Table 1 provides the minimum top widths for dams of various total heights. Total height is the vertical distance between the settled top of the dam and the lowest elevation at the downstream toe.

Design a minimum width of 16 feet for one-way traffic and 26 feet for two-way traffic for the top of dams used as public roads. Design guardrails or other safety measures where necessary and follow the requirements of the responsible road authority. For dams less than 20 feet in total height, maintenance considerations or construction equipment limitations may require increased top widths from the minimum shown in table 1.

Table 1. Minimum top width for dams.

Total height of dam *(feet)*	Top width *(feet)*
Less than 10	6
10–14.9	8
15–19.9	10
20–24.9	12
25–34.9	14
35 or more	15

Side slopes. Design each side slope with a ratio of two horizontal to one vertical or flatter. Design the sum of the upstream- and downstream-side slopes with a ratio of five horizontal to one vertical or flatter. Neither slope shall be steeper than two horizontal to one vertical As required, design benches or flatten side slopes to assure stability of all slopes for all loading conditions.

Slope protection. Design special measures such as berms, rock riprap, sand-gravel, soil cement, or special vegetation as needed to protect the slopes of the dam from erosion. Use NRCS Engineering Technical Release (TR) 210 56, A Guide for Design and Layout of Vegetative Wave Protection for Earth Dam Embankments, and TR-210-59, Riprap for Slope Protection against Wave Action, as applicable.

Freeboard. Design a minimum of 1.0 feet of freeboard between design high-water-flow elevation in the auxiliary spillway and the top of the settled embankment. Design a minimum 2.0 feet of elevation difference between the crest of the auxiliary spillway and the top of the settled embankment when the dam has more than a 20-acre drainage area or more than 20 feet in effective height. Design a minimum of 1.0 feet of freeboard above the peak elevation of the routed design hydrograph to the top of the settled embankment, when the pond has no auxiliary spillway.

Settlement. Increase the height of the dam by the amount needed to ensure that the settled top elevation of the dam equals or exceeds the design top elevation. Design a minimum of 5 percent of the total height of the dam associated with each dam cross section, except where detailed soil testing and laboratory analyses or experience in the area shows that a lesser amount is adequate. Without compaction or wheel track coverage, 10 percent added fill height shall be required.

Principal spillway and pipe conduit through the embankment. Design a pipe conduit with needed appurtenances through the dam, except where rock, concrete, or other types of lined spillways are used, or where a vegetated or earth spillway can safely handle the rate and duration of base flow.

Design a minimum of 0.5-feet difference between the crest elevation of the auxiliary spillway and the crest elevation of the principal spillway when the dam has a drainage area of 20 acres or less. Design a minimum of 1.0-foot difference when the dam has a drainage area of over 20 acres.

Provide an antivortex device for a pipe conduit designed for pressure flow. When design discharge of the principal spillway is considered in calculating peak outflow through the emergency spillway, the crest elevation of the inlet shall be such that the full flow will be generated in the conduit before there is discharge through the emergency spillway. Design the inlet and outlet to function for the full range of flow and hydraulic head anticipated.

Design adequate pipe conduit capacity to discharge long-duration, continuous, or frequent flows without causing flow through the auxiliary spillway. Design a principal spillway pipe with a minimum inside diameter of 4 inches. If the pipe conduit diameter is 10 in or greater, its design discharge may be considered when calculating the peak outflow rate through the emergency spillway. Design pipe with a minimum inside diameter of 1-1/4 inches for water supply pipes to watering facilities or for pipes used for any other purpose.

Design pipe conduits using ductile iron, welded steel, corrugated steel, corrugated aluminum, reinforced concrete (pre-cast or site-cast), or plastic. Do not use cast iron or unreinforced concrete pipe if the dam is 20 feet or greater in total height. Do not use steel or aluminum pipe if the dam is 25 feet or greater in total height. Specifications in tables 2 and 3 are to be followed for polyvinyl chloride (PVC), steel, and aluminum pipe.

Design and install pipe conduits to withstand all external and internal loads without yielding, buckling, or cracking. Design rigid pipe for a positive projecting condition. Design flexible pipe conduits in accordance with the requirements of NRCS National Engineering Handbook (NEH), Part 636, Chapter 52, Structural Design of Flexible Conduits.

Design connections of flexible pipe to rigid pipe or other structures to accommodate differential movements and stress concentrations. Design and install all pipe conduits to be watertight using couplings, gaskets, caulking, water stops, or welding. Design joints to remain watertight under all internal and external loading including pipe elongation due to foundation settlement.

Design a concrete cradle or bedding for pipe conduits if needed to reduce or limit structural loading on pipe and improve support of the pipe.

Design outlet structures, such as cantilever pipe outlet sections and impact basins, to dissipate energy as needed.

Calculate riser and pipe capacities in accordance with procedures set forth in Chapters 3 and 6, Engineering Field Manual for Conservation Practices, 1984 (Rev.). Calculations shall ensure that full pipe flow occurs when the water surface is at the emergency spillway crest elevation. Weir flow shall go directly to pipe flow without experiencing orifice flow at the riser crest or orifice flow at the barrel entrance.

Riser floatation shall be checked and a factor of safety of 1.5 provided against uplift. The weight of submerged soil vertically above the riser base may be used to calculate up-lift balance of forces.

Pipe conduit inlets may be drop inlet spillway, hooded inlet, canopy inlet, siphons, or tee inlet to satisfactorily discharge water from the impoundment. Conduit grades shall be greater than 1 percent but

not steeper than 25 percent. For multiple conduit installations, the minimum spacing between the sides of conduits shall be the greater of 2 feet or 2 times the conduit diameter but need not be greater than 5 feet.

Concrete for risers, riser base, and anti-seep collars shall consist of a mix containing a minimum of 6 bags per cubic yard and a maximum net water content of 7 gallons per bag of cement.

Specifications in tables 2 and 3 are to be followed for polyvinyl chloride (PVC), steel, and aluminum pipe.

Table 2.- Acceptable PVC pipe for use in earth dams[1]

Nominal pipe size	Schedule for standard dimension ratio (SDR)	Maximum depth of fill over pipe
in		*ft*
4 or less	Schedule 40	15
	Schedule 80	20
	SDR 26	10
6,8,10,12	Schedule 40	10
	Schedule 80	15
	SDR 26	10

[1]Polyvinyl chloride pipe, PVC 1120 or PVC 1220, conforming to ATSM-D-1785 or ATSM-D-2241.

Table 3.- Minimum gage for corrugated metal pipe [2-2/3-in x ½-in corrugations][1/]

Fill Height (ft)	Minimum gauge for steel pipe with diameter (in) of — 21 and less	24	30	36	42	48
1 - 15	16	16	16	14	12	10
15 - 20	16	16	16	14	12	10
20 - 25	16	16	14	12	10	10

Fill Height (ft)	Minimum thickness (in) of aluminum pipe[2/] with diameter (in) of — 21 and less	24	30	36
1 - 15	0.06	0.06	0.075	0.075
15 - 20	0.06	0.075	0.105	0.105
20 - 25	0.06	0.105	0.105	---- [3/]

1/ Pipe with 6-, 8-, and 10-in diameters has 1-1/2 in x ¼-in corrugations.
2/ Riveted or helical fabrication.
3/ Not permitted.

Corrosion protection. Provide protective coatings for all steel pipe and couplings in areas that have traditionally experienced pipe corrosion or in embankments with saturated soil resistivity less than 4,000 Ohm-cm or soil pH less than 5. Protective coatings may include asphalt, polymer over galvanizing, aluminized coating, or coal tar enamel.

Ultraviolet protection. Use ultraviolet-resistant materials for all plastic pipe or provide coating or shielding to protect plastic pipe exposed to direct sunlight.

Cathodic protection. Provide cathodic protection for coated welded steel and galvanized corrugated metal pipe where soil and resistivity studies indicate that the pipe needs a protective coating and where the need and importance of the structure warrant additional protection and longevity. If the original design and installation did not include cathodic protection, consider establishing electrical continuity in the form of joint-bridging straps on pipes that have protective coatings. Add cathodic protection later if monitoring indicates the need.

Practice standard 430-FF provides criteria for cathodic protection of welded steel pipe.

Seepage Control (Principle Spillway). Seepage control along a pipe conduit spillway shall be provided if any of the following conditions exit:

1. The effective height of the dam is 15 feet or greater and the effective storage of the dam is 50 acre-ft. or more.

2. A pipe larger than 8 inched in diameter is used that has a smooth exterior wall.

3. The pipe is corrugated and larger than 12 inches in diameter.

Seepage along pipes extending through the embankment shall be controlled by the use of a filter and drainage diaphragm, unless it is determined that anti-seep collars will adequately serve the purpose.

Design the filter diaphragm in accordance with the requirements of NEH, Part 628, Chapter 45, Filter Diaphragms. Locate the filter diaphragm immediately downstream of the cutoff trench, but downstream of the centerline of the dam if the foundation cutoff is upstream of the centerline or if there is no cutoff trench.

To improve filter diaphragm performance, provide a drain outlet for the filter diaphragm at the downstream toe of the embankment. Protect the outlet from surface erosion and animal intrusion.

Ensure filter diaphragm functions both as a filter for adjacent base soils and as a drain for seepage that it intercepts. Materials for the filter diaphragm shall meet the requirements of NEH Part 628, Chapter 45, Filter Diaphragms, Section 628.4503(d), Filter and Drain Gradation.

The drain shall be a minimum of 2 ft thick and extend vertically upward and horizontally at least three times the pipe diameter, and vertically downward at least 18 inches beneath the conduit invert. The drain diaphragm shall be located immediately downstream of the cutoff trench, approximately parallel to the centerline of the dam.

When using anti-seep collars in lieu of a filter diaphragm, ensure a watertight connection to the pipe. Limit the maximum spacing of the anti-seep collars to 14 times the minimum projection of the collar measured perpendicular to the pipe, or 25 feet, whichever is less. Locate anti-seep collars no closer than 10 feet apart. Use a collar material that is compatible with the pipe material.

When using anti-seep collars, design the collars to increase the seepage path along the pipe within the fill by at least 15 percent.

Trash guard. Install a trash guard at the riser inlet to prevent clogging of the conduit, unless the watershed does not contain trash or debris that could clog the conduit.

Pool Drain. Provide a pipe with a suitable valve to drain the pool area if needed for proper pond management or if required by State law. The designer may use the principal spillway conduit as a pond drain if it is located where it can perform this function.

Auxiliary spillways. A dam must have an open channel auxiliary spillway, unless the principal spillway is large enough to pass the peak discharge from the routed design hydrograph and the trash that comes to it without overtopping the dam. The minimum criteria for acceptable use of a closed conduit principal spillway without an auxiliary spillway consist of a conduit with a cross-sectional area of 3 feet^2 or more, an inlet that will not clog, and an elbow designed to facilitate the passage of trash.

Design the minimum capacity of a natural or constructed auxiliary spillway to pass the peak flow expected from a total design storm of the frequency and duration shown in table 4, less any reduction creditable to the conduit discharge and detention storage.

Table 4. Minimum auxiliary spillway capacity

Drainage area *(acre)*	Effective height of dam[1] *(feet)*	Detention storage *(acre-feet)*	Minimum design storm[2]	
			Frequency *(years)*	Minimum duration *(hours)*
20 or less	20 or less	< than 50	10	24
20 or less	> than 20	< than 50	25	24
> than 20	-	< than 50	25	24
All others	-	-	50	24

[1]. Defined under "Conditions where Practice Applies."

[2]. Select rain distribution based on climatological region.

Discharge and detention storage reductions may be used for ponds having a 10-inch diameter or larger conduit with 1) computer flood routing procedures or 2) shortcut flood routing method from Chapter 11, Engineering Field Manual (EFW) for Conservation Practices. The minimum emergency spillway capacity for ponds without conduit and ponds with conduit less than 10 inches in diameter shall be in accordance with procedures in Chapter 11, EFM or with the following table:

Table 5. Minimum storage and spillway capacity. 1/

Peak 2/ Runoff	Type Spillway	Floodwater Storage	Bottom Width 3/			
			Hp = 1.0 V = 3.5 fps	Hp = 1.1 V = 4.0 fps	Hp = 1.2 V = 4.5 fps	Hp = 1.3 V = 5.0 fps
0-25 cfs	Veg.	0	16	12	10	10
25-50 cfs	Veg.	0	32	24	18	16
50-75 cfs	Veg.	0	56	42	34	28
75-100 cfs	Veg. & conduit	1' stage	60	44	36	30
100-150 cfs	Veg. & conduit	1' stage	98	74	60	50
150-200 cfs	Veg. & conduit	1' stage	--	92	72	60
200+	Floodrouting procedures required for all pipe conduit and vegetative spillways.					

1/ Applicable to ponds having less than 50 AF total storage to the earth spillway crest. Ponds having more than 20 acres drainage area shall have a conduit spillway.

2/ Peak runoff computed in accordance with EFM Chapter 2 procedures and Table 6 requirements. A 15 percent peak Q reduction has been included in pond with a pipe spillway.

3/ For stable vegetative spillway conditions only and exit slopes between 2 percent and 7 percent. When an erosive soil condition, exit channel slope drop off, or poor cover condition exists in the vegetative spillway outlet, a conduit and temporary detention storage will be provided for the entire applicable storm runoff.

Design the auxiliary spillway to safely pass the peak flow through the auxiliary spillway, or route the storm runoff through the reservoir. Start the routing either with the water surface at the elevation of the crest of the principal spillway or at the water surface after 10 days' drawdown, whichever is higher. Compute the 10-day drawdown from the crest of the auxiliary spillway or from the elevation attained from impounding the entire design storm, whichever is lower. Design the auxiliary spillway to pass the design flow at a safe velocity to a point downstream where the flow will not endanger the dam.

A constructed auxiliary spillway consists of an inlet channel, a control section, and an exit channel. Design the auxiliary spillway with a trapezoidal cross section. Locate the auxiliary spillway in undisturbed or compacted earth or in-situ rock. Design for stable side slopes for the material in which the spillway is to be constructed. Design a minimum bottom width of 10 feet for dams having an effective height of 20 feet or more.

Design a level inlet channel upstream from the control section for the distance needed to protect and maintain the crest elevation of the spillway. If necessary, curve the inlet channel upstream of the level section to fit existing topography. The emergency spillway control section shall have a level grade for a minimum distance of 10 feet, 25 feet for drainage areas exceeding 50 acres. Design the exit channel grade in accordance with NEH Part 628, Chapter 50, Earth Spillway Design, or with equivalent procedures.

Structural auxiliary spillways. When used for principal spillways or auxiliary spillways, design chute spillways or drop spillways according to the principles set forth in NEH, Part 650, Engineering Field Handbook; and NEH, Section 5, Hydraulics; Section 11, Drop Spillways; and Section 14, Chute Spillways. Design a structural spillway with the minimum capacity required to pass the peak flow expected from a total design storm of the frequency and duration shown in table 4, less any reduction creditable to the conduit discharge and detention storage.

Criteria for Excavated Ponds

Runoff. Design a minimum of 1.0 feet of freeboard above the peak elevation of the routed design hydrograph. Design a pipe and auxiliary spillway that will meet the capacity requirements of table 4. Consider runoff flow patterns when locating the excavated pond and placing the spoil.

Side slopes. Design stable side slopes in the excavated area no steeper than one horizontal to one vertical.

Watering Ramp. If livestock will water directly from the pond, a watering ramp of ample width shall be provided. The ramp shall extend to the anticipated low water elevation at a slope no steeper than three horizontal to one vertical.

Inlet protection. Protect the side slopes from erosion where surface water enters the pond in a natural or excavated channel.

Excavated material. Place the material excavated from the pond so that its weight does not endanger the stability of the pond side slopes and so that the soil will not wash back into the pond by rainfall. Dispose of excavated material in one of the following ways:

- Uniformly spread to a height that does not exceed 3 feet, with the top graded to a continuous slope away from the pond.
- Uniformly place or shape reasonably well, with side slopes assuming a natural angle of repose. Place excavated material at a distance equal to the depth of the pond, but not less than 12 feet from the edge of the pond.
- Shape to a designed form that blends visually with the landscape.
- Provide for low embankment construction and leveling of surrounding landscape.
- Haul material offsite.

Pond Clean-out

Reduced water volume capacity of existing ponds, excavated or embankment types, having a surface area of 0.2 acres or less, shall be restored by removal of accumulated sediment to the extent practical. During the sediment removal process, steps shall be taken to maintain stable side slopes. All excavated sediment shall be placed in accordance with the criteria for excavated material section of this standard, unless otherwise approved by the local or other appropriate engineer. After clean-out, the pond shall be in compliance with the requirements of this standard.

Additionally, for embankment ponds, any work determined to be necessary by the appropriate engineer shall be performed to meet the requirements of the standard. This work may include addressing principle and/or auxiliary spillways and levee slopes.

CONSIDERATIONS

Visual resource design. Carefully consider the visual design of ponds in areas of high public visibility and those associated with recreation. The shape and form of ponds, excavated material, and plantings are to relate visually to their surroundings and to their function.

Shape the embankment to blend with the natural topography. Shape the edge of the pond so that it is generally curvilinear rather than rectangular. Shape excavated material so that the final form is smooth, flowing, and fitting to the adjacent landscape rather than angular geometric mounds. If feasible, add islands to provide visual interest and to attract wildlife.

Fish and wildlife. Locate and construct ponds to minimize the impacts to existing fish and wildlife habitat.

When feasible, retain structures such as trees in the upper reaches of the pond and stumps in the pool area. Shape upper reaches of the pond to provide shallow areas and wetland habitat.

If operations include stocking fish, use CPS Code 399, Fishpond Management.

Vegetation. All exposed areas of the embankment, earth spillway, and borrow area(s) shall be protected with vegetation. Vegetation will be applied in accordance with Conservation Practice Standard 342, Critical Area Planting.

Water quantity. Consider effects upon components of the water budget, especially—

- Effects on volumes and rates of runoff, infiltration, evaporation, transpiration, deep percolation, and groundwater recharge.
- Variability of effects caused by seasonal or climatic changes.
- Effects on downstream flows and impacts to environment such as wetlands, aquifers, and social and economic impacts to downstream uses or users.

Water quality. Consider the effects of—

- Erosion and the movement of sediment, pathogens, and soluble and sediment-attached substances that runoff carries.
- Short-term and construction-related effects of this practice on the quality of downstream watercourses.
- Water-level control on the temperatures of downstream water to prevent undesired effects on aquatic and wildlife communities.
- Wetlands and water-related wildlife habitats.
- Water levels on soil nutrient processes such as plant nitrogen use or denitrification.
- Soil water level control on the salinity of soils, soil water, or downstream water.
- Potential for earth moving to uncover or redistribute toxic materials.

- Livestock grazing adjacent to the pond. Consider fencing to keep livestock activities out of direct contact with the pond and dam.

PLANS AND SPECIFICATIONS

Prepare plans and specifications that describe the requirements for applying the practice according to this standard. As a minimum, include the following items:

- A plan view of the layout of the pond and appurtenant features
- Typical profiles and cross sections of the principal spillway, auxiliary spillway, dam, and appurtenant features as needed
- Structural drawings adequate to describe the construction requirements
- Requirements for vegetative establishment and/or mulching, as needed
- Safety features
- Site-specific construction and material requirements

OPERATION AND MAINTENANCE

Prepare an operation and maintenance plan for the operator.

As a minimum, include the following items in the operation and maintenance plan:

- Periodic inspections of all structures, earthen embankments, spillways, and other significant appurtenances
- Prompt repair or replacement of damaged components
- Prompt removal of sediment when it reaches predetermined storage elevations
- Periodic removal of trees, brush, and undesirable species
- Periodic inspection of safety components and immediate repair if necessary
- Maintenance of vegetative protection and immediate seeding of bare areas as needed

REFERENCES

American Society for Testing and Materials. Standard Practice for Classification of Soils for Engineering Purposes (Unified Soil Classification System), ASTM D2487. West Conshohocken, PA.

USDA NRCS. Engineering Technical Releases, TR-210-60, Earth Dams and Reservoirs. Washington, DC.

USDA NRCS. National Engineering Handbook (NEH), Part 628, Dams. Washington, DC.

USDA NRCS. NEH, Part 633, Soil Engineering. Washington, DC.

USDA NRCS. NEH, Part 636, Structural Engineering. Washington, DC.

USDA NRCS. NEH, Part 650, Engineering Field Handbook. Washington, DC.

USDA NRCS. National Engineering Manual. Washington, DC.

Natural Resources Conservation Service
Construction Specifications

POND

1. SCOPE

This scope of work consists of constructing the pond and includes all clearing, excavation, fill placement, installation of a pipe spillway, drains, and other features to lines, grades, and elevations as specified on the drawings and staked in the field. See the location of the embankment as shown on furnished drawings or as staked in the field.

2. SITE PREPARATION

Clear the pool area to the extent desired and as shown on the plans. Cut trees as flush with the ground as practical and burned, buried at designated locations, or anchored to the pond bottom.

The clearing of the staked foundation, spillway, and borrow area(s) includes removal of logs, stumps, roots, sod, and other trash that would prevent a good bond between the foundation and fill material.

3. EXCAVATION

Stockpile topsoil from foundation, emergency spillway, and borrow area(s) for spreading on the completed dam and spillway as needed to help establish vegetation.

Deepen existing stream channels crossing the foundation area and widened as necessary to remove all stones, gravel, sand, sediment, stumps, roots, organic matter and other objectionable material and to accommodate compaction equipment. Construct side slopes of the channels and other foundation surfaces to no steeper than 1:1. Thoroughly scarify the foundation area before placement of the fill material. Add moisture and compact soil as necessary so that the first layer of fill material will be bonded to the foundation.

Excavate the cutoff trench to the depths, bottom width and side slopes shown on the plans. Material removed from the cutoff trench which is free of boulders, roots, organic matter and other objectionable material may be placed in the downstream one-third of the fill. Remove all excess water from the trench and the foundation area when fill material is being placed.

Construct excavated ponds to conform to the shapes, lines and grades shown on the drawings or as staked in the field. Place the excavated material from the pond so that its weight will not endanger the stability of the pond side slopes and so that it will not be washed back into the pond by rainfall.

4. FILL PLACEMENT

Ensure the material placed in the fill is free of all sod, roots, frozen soil, stones over six inches in diameter, and other objectionable material.

Begin placing and spreading of the fill material at the lowest point of the foundation and bring up in approximately horizontal layers not exceeding eight inches thick. Ensure these layers are reasonably uniform in thickness and it extends over the entire area of the fill. Operate the earth hauling or compacting equipment over each layer so that reasonable compaction of the fill material will be obtained. Add a minimum of 5 percent to fill heights constructed with compaction equipment or having each layer covered by the wheel track of construction equipment during the fill placement process. Without compaction or wheel track coverage, add 10 percent to the fill height.

Ensure the distribution and gradation of materials throughout the fill has no lenses, pockets, streaks, or layers of material differing substantially in texture or gradation from the surrounding material. Where it is necessary to use material of varying texture and gradation, place the more impervious material in the upstream and center portions of the fill. Undertake construction of the fill only at such times when the moisture content of the fill material will permit a satisfactory degree of compaction and bonding or when moisture can be satisfactorily added and incorporated in dry soil material as it is being placed. Finish the embankment, emergency spillway, and borrow areas to a smoothness so the surface can be readily traveled upon by farm type equipment.

Final construction is considered satisfactory when:

Excavation elevations are within $\pm$ 0.2 foot of design grade or modified grade. Excavation slopes may be flatter than designed but not steeper.

Fill elevations are not less than design height plus settlement. Fill above the required settlement elevation will require extra fill material to maintain side slopes within design limits.

Fill slopes may be flatter than designed but may not be steeper and are uniform throughout their length. Allowance for anticipated settlement may be considered when calculating side slopes for construction check.

Place selected backfill material around structures, pipe conduits, and drain-fill or anti-seep collars at about the same rate on all sides to prevent damage from unequal loading. Compact the fill adjacent (within one foot) to these components to a density equivalent to that of the surrounding fill by hand tamping or by using manually directed power tampers or plate vibrators. Take care during backfill around pipe conduit to prevent uplift of pipe. Preparation of a shaped bed with one-inch of moist, loose soil supporting about one-third of pipe circumference will help ensure the pipe to soil contact.

Keep the drain-fill material placed next to the pipe conduit or other structural features free of contaminating fill materials by either placing it in a cleanly excavated trench or by keeping the drain-fill at least one foot above the adjacent earthfill.

5. CONSTRUCTION MATERIALS

Ensure the pipe conduit conforms to appropriate ASTM and federal specifications. Verify anti-seep collars are fabricated of materials compatible with that of the pipe and are installed so that they are water tight. Install the pipe according to the manufacturer's instructions and ensure that it is firmly and uniformly bedded throughout its length to the specified line and grade shown on the drawings.

Utilize all new material for the components needed for the construction. Ensure the material is of good quality, free of dents, deformities or other issues that might reduce the durability, capacity, or planned life of the structural measure.

Consider spillway conduit installation satisfactory when the conduit is within $\pm$ 0.2 foot of design grade, has a positive slope toward the outlet, has the required appurtenances (bands, anti-seep collars, risers, cathodic protection, etc.) attached, has all surface coating damage repaired, and has adequate backfill and compaction applied.

For the concrete used on anti-seep collars, riser base, riser crest perimeter protection, or pipe inlet protection, use a mix containing a minimum of six bags per cubic yard and a maximum net water content of seven gallons per bag of cement. Allow a minimum 24 hours curing time before fill material is placed against the concrete. Place and finish the concrete in an acceptable manner. Place reinforcing steel as indicated on the plans and hold it securely in place during concrete placement. Install subgrades and forms to line and grade, and ensure the forms are mortar tight and unyielding as the concrete is placed.

Ensure the filter and drainage diaphragm materials have a gradation equal to the fine concrete aggregate gradation listed in the Mississippi State Highway Department, "Standard Specifications for Road and Bridge Construction." Install the drain with a minimum 2-foot thickness of uncontaminated filter material.

6. VEGETATIVE PROTECTION

Plant all the exposed surfaces of the embankment, spillway, excavation disposal area, and borrow area(s) to establish a vegetative protective cover. Utilize Conservation Practice Standard 342, Critical Area Planting to determine the needed species, seeding rates, fertilizer, lime, etc. for vegetation establishment. Temporary vegetation may be used until permanent vegetation can be established. Fence the embankment and spillway if necessary to protect vegetation from livestock.

Provide surface drainage around excavation disposal areas and other areas for non-erosive entry of runoff into the pond or for non-erosive disposal of runoff away from the pond embankment.

7. VEGETATION DETAILS

Seeding Date: _____ _____ _____

Seed Type: _____ _____ _____

Seed Rate: _____ _____ _____

Soil Preparation: _____

Fertilizer: _____Rate _____pounds/acre

Lime: _____Rate _____pounds/acre

Mulch: _____Rate _____pounds/acre

Lime: _____Rate _____pounds/acre

Other: _____

8. CONSTRUCTION DETAILS

_____

_____

_____