



Natural Resources Conservation Service
CONSERVATION PRACTICE STANDARD
WASTE STORAGE FACILITY

Code 313
(Number)

DEFINITION

An agricultural waste storage impoundment or containment made by constructing an embankment, excavating a pit or dugout, or by fabricating a structure.

PURPOSE

To store manure, agricultural by-products, wastewater, and contaminated runoff to provide the agricultural operation management flexibility for waste utilization.

CONDITIONS WHERE PRACTICE APPLIES

Use where regular storage is needed for wastes generated by agricultural production or processing and where soils, geology, and topography are suitable for construction of the facility. For reception pits, use the NRCS Conservation Practice Standard (CPS) Waste Transfer (Code 634).

For liquid waste storage facilities implemented with an embankment, this practice applies only to low hazard structures as defined in the NRCS National Engineering Manual (NEM), Part 520.23.

This practice does not apply to the storage of human waste or routine animal mortality.

CRITERIA

General Criteria Applicable to All Waste Storage Facilities.

Laws and Regulations. Plan, design, and construct the waste storage facility to meet all Federal, State, and local laws and regulations. [These include Vermont Required Agricultural Practices \(RAP's\) and Large, Medium and Small Farm Operation Regulations.](#)

Location. Locate and design the waste storage facility such that it is outside the 100-year floodplain unless site restrictions require locating it within the floodplain. If located in the floodplain, protect the facility from inundation or damage from a 25-year flood event. Additionally, follow the policy found in the NRCS General Manual (GM) 190, Part 410.25, Flood Plain Management, which may require providing additional protection for storage structures located within the floodplain.

Waste storage facilities shall be located a minimum set back distance from:

Resource Concern	Up Gradient	Down Gradient
• Well Spring or other Potable Water Source	300 Feet	200 Feet
• Neighboring Wells	500 Feet	500 Feet
• Centerline of Public Roads	100 Feet	100 Feet
• Abutting Property Line	100 Feet	100 Feet
• Top of Bank from any Body of Water	200 Feet	200 Feet

Increase these distances in environmentally sensitive locations such as Well Head Protection Areas.

Storage Period. The storage period is the maximum length of time anticipated between emptying events. Base the minimum storage period on the timing required for environmentally safe waste utilization considering the climate, crops, soil, equipment, and local, State, and Federal regulations. The minimum storage duration shall be no less than 180 days. However, longer storage periods are recommended and shall be compatible with the planned manure application schedule per the nutrient management plan. Storage periods greater than 270 days must be approved by the State Conservation Engineer.

Design Storage Volume. Size the facility to store the following volumes as appropriate.

Operational Volume

- Manure, wastewater, bedding, and other wastes accumulated during the storage period.
- For liquid or slurry storage facilities, include normal precipitation (omit diverted roof runoff) less evaporation during the storage period.
- Normal runoff from the facility's drainage area during the storage period.
- Planned maximum residual solids. Provide a minimum of 6 inches for tanks and other structures and 18 inches for manure ponds, unless provisions are made to remove the residual solids on an annual basis.
- Additional storage when required to meet management goals or regulatory requirements.

Emergency Volume (liquid storages only)

- 25-year, 24-hour precipitation on the surface of the liquid or slurry storage facility at the maximum level of the required design storage.
- 25-year, 24-hour runoff from the facility's drainage area.

Freeboard Volume (for liquid or slurry waste storage exposed to precipitation)

- Minimum of 6" for vertical walled tanks and other fabricated structures.
- Minimum of 12" for all other facilities.

Exclude nonpolluted runoff from the structure to the fullest extent practical except where including the runoff is advantageous to the operation of the agricultural waste management system.

Inlet. Design inlet to resist corrosion, plugging, freeze damage, and ultraviolet deterioration. Incorporate erosion protection as necessary. Design inlets, such as gravity pipes and pump transfer systems in accordance to practice code 634 – "Waste Transfer".

Picket Dams and Perforated Risers. Picket dams and perforated risers may be used on uncovered manure stacking facilities for draining rain water, snow melt and liquid manure. The drainage water shall be collected and directed to a storage or treatment facility. Picket dams shall be designed to withstand anticipated hydrostatic loads imposed by the manure. The openings between pickets shall be 3/4 to 1 inch wide. Perforated risers shall be located out of the way, braced or protected from damage by unloading equipment or manure loads. Riser openings shall be 3/4 inch to 1-inch holes or slots randomly spaced around the full height of the riser. Screens may be installed with perforated risers to reduce plugging of perforations. Picket dams and perforated risers may be used together.

Waste Removal. Provide components for removing waste such as gates, pipes, docks, wet wells, pumping platforms, retaining walls, or ramps. Incorporate features to protect against erosion, tampering, and accidental release of stored waste as necessary. Design ramp slopes to accommodate anticipated equipment and traction available. Use NRCS CPS Nutrient Management (Code 590) for land application of stored material or follow other disposal options outlined in a Comprehensive Nutrient Management Plan (CNMP). [Install anti-scour pads at all planned pump out and agitation locations to prevent erosion or damage to the foundation, embankment or lining of the waste storage facility.](#)

Accumulated Solids Removal. To preserve storage volume, make provision for periodic removal of accumulated solids. The anticipated method for solids removal must be accommodated in design, particularly in determining the configuration of impoundments and the type of liner to be used.

Maximum Operating Level. The maximum operating level for liquid storage structures is the level that provides the operational volume.

Staff Gauge. Place a staff gauge or other permanent marker in the liquid storage facility to clearly indicate the following elevations:

- Maximum operating level (top of the operational volume).
- Emergency level (top of the design storage volume).

For storages where the contents are not visible and a staff gauge would not be visible, such as below a slatted floor, identify the method for the operator to measure the depth of accumulated waste in the Operation and Maintenance Plan.

Safety. Include appropriate safety features to minimize the hazards of the facility (refer to American Society of Agricultural and Biological Engineers (ASABE) Standard EP470, Manure Storage Safety for guidance, as needed).

Provide warning signs, fences, ladders, ropes, bars, rails, and other devices as appropriate, to ensure the safety of humans and livestock. Provide ventilation and warning signs for covered waste holding structures, as necessary, to prevent explosion, poisoning, or asphyxiation. [Post warning signs at all reception pits, hoppers and any other confined area that may contain harmful gases.](#)

Design covers and grating over openings such that livestock or humans cannot accidentally displace them and fall into the facility.

Design pipelines with a water-sealed trap and vent, or similar device, if there is a potential for gases from the pipe to accumulate in confined spaces.

Place a fence around impoundments and uncovered tanks which have exposed walls less than 5 feet above ground surface. Use the NRCS CPS Fence (Code 382) for design of a fence and appurtenances that will prevent accidental entry by people or animals likely to be onsite. Except at push-off and unloading locations, fencing around waste storage facility shall be one of the following:

- Woven Wire (6 inch grid) with a single strand of barbed wire above,
- Five strand barbed wire,
- Six strand high tensile, or
- Chain Link.

No other fencing style or configuration will be allowed without the State Conservation Engineer's approval. All fences installed around waste storage facilities shall be a minimum of 48 inches high from the ground to the top of the wire. At all push-off locations, install safety features such as a bar, rail, chain or other type of barrier to prevent equipment from accidentally falling into the facility. At all agitation and unloading locations, install a minimum of a six rail pipe gate which needs to be kept closed except during unloading activities.

Post a minimum of four universal warning signs to prevent children and others from entering liquid waste storage structures.

Roofs and Covers. Use NRCS CPS Roofs and Covers (Code 367) for design of waste storage facility covers or roofs, as needed.

Treated Wood. Use criteria from NRCS CPS Roof and Covers (Code 367) for treated wood and fasteners.

Additional Criteria for Liquid Waste Storage Impoundments

A liquid waste storage impoundment is a facility where the stored material does not consistently stack and is either a natural topographic depression, manmade excavation, or diked area formed primarily of earthen materials, such as soil (although the unit may be lined with manmade materials) .

Foundation. Locate the impoundment in soils with a permeability that meets all applicable regulations or line the impoundment with suitable material. Use liners which meet or exceed:

- CPS 520 – Pond Sealing or Lining, Compacted Soil Treatment
- CPS 521 – Pond Sealing or Lining, Geomembrane or Geosynthetic Clay Liner
- CPS 522 – Pond Sealing or Lining, Concrete using “Liquid Tight” criteria

Perform subsurface investigations for all waste storage impoundments sufficient in detail and analysis to support the design in accordance with NRCS NEM, Part 531, Geology. Describe the soil material encountered, location of any seeps, depth-to-high-water table, depth to bedrock, and presence of sink holes in karst topography.

For the design of a liner on a site located in a floodplain and other locations where there is potential for uplift, include an evaluation of all potential buoyant uplift forces on the liner. Limit projected uplift head under clay liners to a gradient of less than 0.5 ft/ft in the clay liner. The gradient is determined as the difference in total head between the top and the bottom of a clay liner when buoyant forces exist (such as when the floodplain is flooded) divided by the thickness of the clay liner.

Design Bottom Elevation. Locate the impoundment bottom elevation a minimum of 2 feet above the seasonal high water table unless special design features are incorporated that address buoyant forces, impoundment seepage rate and nonencroachment of the water table by contaminants. The water table may be lowered by use of drains to meet this requirement. Locate the impoundment bottom elevation a minimum of 2 feet above bedrock regardless of lining type. Blasting of bedrock within 300 feet of an on-farm water source, i.e. well, spring, etc., must have State Conservation Engineer's approval.

Outlet. [Installation of gravity outlet structures of any kind shall not be permitted.](#) An outlet that can automatically release stored material is not permitted except for septic tanks that feed a treatment system such as a waste treatment strip or leaching field or outlets leading to another storage facility with adequate capacity. Design a permanent outlet that will resist corrosion and plugging. Provide a backflow prevention measure for an outlet that pumps wastewater to secondary storage located at a higher elevations.

Embankments. For an impoundment with greater than one acre of surface area and where wave action is a concern, increase the embankment height to account for calculated wave height. In all cases, increase the constructed embankment height by at least 5 percent to allow for settlement. Stabilize all embankments to prevent erosion or deterioration.

Minimum embankment top widths are shown in Table 1. Design the combined side slopes of the settled embankment to be equal to or flatter than 5 horizontal to 1 vertical, with neither slope steeper than 2 horizontal to 1 vertical unless provisions are made for stability. The total embankment height (effective height) is the difference in elevation between the auxiliary (emergency) spillway crest or the settled top of the embankment if there is no auxiliary spillway and the lowest point in the cross section taken along the centerline of the embankment.

Table 1 Minimum Top Widths

Total embankment height (ft)	Top width (ft)
Less than 15	8
15–19.9	10
20–24.9	12
25–30	14
30–35	15

Spillway or Equivalent Protection. For a facility having a total embankment height greater than 20 feet, construct an auxiliary (emergency) spillway or route through the spillway or store below the spillway another volume equivalent to the emergency volume.

Excavations. Design excavated side slopes to meet the requirements of the liner used, see NRCS CPS Pond Sealing or Lining, Compacted Soil Treatment (Code 520), Pond Sealing or Lining, Flexible Membrane (Code 521a) or Pond Sealing or Lining, Concrete (Code 522), .

Additional Criteria for Fabricated Structures

[A fabricated structure is a facility where the stored material does not consistently stack and is primarily constructed of walls made of concrete, timber, glass lined steel or other non-earthen materials.](#)

Foundation. Based on subsurface investigation, provide a foundation for fabricated waste storage structures to safely support all superimposed loads without excessive movement or settlement. Perform subsurface investigations for all fabricated structures sufficient in detail and analysis to support the design in accordance with NRCS NEM, Part 531, Geology. Describe the soil material encountered, location of any seeps, depth to high water table, depth to bedrock, and presence of sink holes in karst topography. [Blasting of bedrock within 300 feet of an on-farm water source, i.e. well, spring, etc., must have State Conservation Engineer's approval.](#)

Where a nonuniform foundation cannot be avoided or where applied loads may create highly variable foundation loads, calculate settlement based upon site-specific soil test data. Index tests of site soil may allow correlation with similar soils for which test data is available. If no test data are available, use presumptive bearing strength values for assessing actual bearing pressures obtained from Table 2 or

another nationally recognized building code. In using presumptive bearing values, provide adequate detailing and articulation to avoid distressing movements in the structure.

For bedrock foundations with joints, fractures, or solution channels, separate the floor slab and the bedrock by—

- A minimum of 1 foot of soil.
- A liner that meets or exceeds NRCS CPS Pond Sealing or Lining (Codes 520, 521, or 522).
- Other appropriate method or alternative that achieves equal protection.

Table 2 Presumptive Allowable Foundation and Lateral Pressure¹

Class of materials	Allowable foundation pressure (psf)	Lateral bearing (psf/ft) below natural grade	Coefficient of friction	Cohesion (psf)
Crystalline bedrock	12,000	1,200	0.70	-
Sedimentary and foliated rock	4,000	400	0.35	-
Sandy gravel or gravel (GW and GP)	3,000	200	0.35	-
Sand, silty sand, clayey sand, silty gravel, clayey gravel (SW, SP, SM, SC, GM and GC)	2,000	150	0.25	-
Clay, sandy clay, silty clay, clayey silt, silt and sandy silt (CL, ML, MH and CH)	1,500	100	-	130

¹ International Building Code (IBC), 2015, International Code Council (ICC)

Structural Loadings. Design the waste storage structure to withstand all anticipated loads in accordance with the requirements in NRCS NEM, Part 536, Structural Design. Such loads should include internal and external loads, hydrostatic uplift pressure, concentrated surface and impact loads, and water pressure due to seasonal high water table, frost or ice.

Calculate loading from lateral earth pressures using soil strength values determined from the results of appropriate soil tests and procedures described in Technical Release 210-74, Lateral Earth Pressures. Table 3 provides minimum lateral earth pressure values when soil strength tests are not available. If heavy equipment will operate near the wall, use an additional soil surcharge or an additional internal lateral pressure in the wall analysis as appropriate.

For the lateral load from stored waste not protected from precipitation, use a minimum 65 psf/ft of depth as the design internal lateral pressure. Use a minimum value of 60 psf/ft of depth for the lateral load from stored waste protected from precipitation and not likely to become saturated. Use a minimum internal lateral pressure of 72 psf/ft of depth for sand-laden manure storage if the percentage of sand exceeds 20%. Designers may use lesser values if supported by measurement of actual pressures of the waste to be stored.

Table 3 Minimum Lateral Earth Pressure Values¹

Description of backfill material ^c	Unified soil classification	Design lateral soil load (psf/ft of depth) ^a	
		Active pressure	At-rest pressure
Well-graded, clean gravels; gravel-sand mixes	GW	30	60
Poorly graded clean gravels; gravel-sand mixes	GP	30	60
Silty gravels, poorly graded gravel-sand mixes	GM	40	60
Clayey gravels, poorly graded gravel-sand mixes	GC	45	60
Well-graded, clean sands; gravelly sand mixes	SW	30	60
Sand-silt clay mix with plastic fines	SP	30	60
Silty sands, poorly graded sand-silt mixes	SM	45	60
Sand-silt clay mix with plastic fines	SM-SC	45	100
Clayey sands, poorly graded sand-clay mixes	SC	60	100
Inorganic silts and clayey silts	ML	45	100
Mixture of inorganic silt and clay	CL-ML	60	100
Inorganic clays of low to medium plasticity	CL	60	100
Organic silts and silt clays, low plasticity	OL	Note ^b	Note ^b
Inorganic clayey silts, elastic silts	MH	Note ^b	Note ^b
Inorganic clays of high plasticity	CH	Note ^b	Note ^b
Organic clays and silty clays	OH	Note ^b	Note ^b

¹ Table 1610.1, Lateral Soil Load, International Building Code (IBC), 2015, International Code Council (ICC).

^a Design loads based on moist conditions for the specified soils at optimum density. Include the weight of the buoyant soil plus hydrostatic pressure for submerged or saturated soil.

^b Unsuitable as backfill material.

^c Base the definition and classification of soil in accordance with ASTM D 2487.

Structural Design. Design structures with reinforced concrete, steel, wood, or masonry materials in accordance with NRCS-NEM, Part 536, Structural Engineering. Account for all items that will influence the performance of the structure, including loading assumptions, durability, serviceability, material properties and construction quality. Ensure that the material used for a fabricated structure is compatible with the waste product to be stored.

Tanks may be designed with or without a cover. Design openings in a covered tank to accommodate equipment for loading, agitating, and emptying. Equip these openings with fencing, grills or secure covers for safety, and for odor and vector control as necessary.

Sensitive Environmental Settings. Where liquid-storage is to be provided in sensitive environmental settings (i.e., tanks in areas with shallow wells in surface aquifers, high-risk karst topography, or other site-specific concerns), design the storage structure as a reinforced concrete hydraulic or environmental structure according to NRCS NEM, Part 536, Structural Design. Alternatively, use a flexible liner membrane, designed and constructed in accordance with standard engineering and industry practice, to provide secondary liquid containment for structures constructed with other methods described in NRCS NEM, Part 536, Structural Design.

Non-Sensitive Environmental Settings. All Concrete Waste Storage Structures shall be design according to the current ACI 318 – Building Code Requirements for Structural Concrete with the following additional criteria:

1. The minimum concrete compressive strength, f_c , shall be 4000 psi, with a maximum design & placed, water to cement ratio, $w/C_{max} = 0.45$.
2. The maximum reinforcing steel yield strength, f_y , shall be Grade 60, for deformed bars.
3. The maximum spacing of reinforcing bars, in both orthogonal directions, shall be 12”.
4. Minimum concrete cover over reinforcing steel shall be according to Table 4:

Table 4 Minimum Cover condition for Environmental Structures and Ag. Waste Structures

Cover Condition	Minimum Cover (Inches)
(a) Concrete cast against and permanently exposed to earth	3
(b) Concrete exposed to earth, liquid, weather or cast against a work mat:	
• Slab and Joist	2
• Beam and Columns:	
○ Stirrups, spirals and ties	2
○ Primary Reinforcement	2
• Footings and Base Slabs:	
Formed Surfaces	2
Top of Footing and Base Slabs	2

5. The minimum temperature and shrinkage reinforcement ratios (steel area to gross concrete area), for all concrete sections; walls, footings & slabs, in both orthogonal directions, shall be according to Table 5:

Table 5 Minimum Temperature and Shrinkage and Temperature Reinforcement for Environmental Structures

Length between Movement Joints (Feet)	Minimum Shrinkage and Temperature Reinforcement Ratio	
	Grade 40	Grade 60
Less than 20	0.0030	0.0030
20 to less than 30	0.0040	0.0030
30 to less than 40	0.0050	0.0040
40 and greater	0.0060*	0.0050*

* To be used when no movement joints are provided unless analysis indicates a greater amount is required.

6. Splices in hoop steel of non prestressed circular concrete tanks shall be staggered 3 feet horizontally for a minimum of three rows of reinforcing. May repeat starting at the fourth row.

7. Load combinations and factors shall be according to the current ACI 318 – Building Code Requirements for Structural Concrete, with the addition of an environmental durability factor S_d , as follows:

Table 6 Standard Environmental Durability Factors for Flexure and Shear

Load Case	Environmental Durability Factor, S_d	
	Flexure $f_s = 36$ ksi $\phi = 0.90$	Direct Tension $f_s = 30$ ksi $\phi = 0.90$
Dead or Fluid ($\gamma=1.4$)*	1.07	1.286
Dead or Fluid ($\gamma=1.2$)**	1.25	1.50
Live or Soil ($\gamma=1.6$)	1.00	1.125

* Acting alone

** Acting in combination with other loads

8. Maximum steel stresses from unfactored loads shall be as follows:
- Flexural tensile stress, $f_s = 36$ ksi
 - Direct tensile stress, $f_s = 30$ ksi
 - Shear reinforcement tensile stress, $f_s = 36$ ksi
9. Maximum steel ratio with Grade 60 reinforcing shall be $0.3\rho_b$
10. Non-structural slabs on grade shall be designed according to the current ACI 350 Appendix H.
- Minimum $f_c = 4000$ psi
 - Maximum $w/c = 0.45$
 - Minimum reinforcing ratio in both orthogonal directions shall be 0.005 (0.5%) for slabs with cold joints spaced 40' and greater. Other ratios according to Table 5 Minimum Temperature and Shrinkage and Temperature Reinforcement for Environmental Structures Table 5.
 - Maximum spacing of deformed bars shall be the lesser of 12 inches or two times the slab thickness.
 - Maximum wire spacing of WWF shall be 4 inches.
 - Slabs shall be placed in sections as large as possible to limit the number of cold joints. Dry and temperature shrinkage shall be considered.
 - All cold joints shall have an approved waterstop, installed according to the manufacturer's instructions. Ensure minimum concrete cover requirements are met.

Additional Criteria - Stacking Facilities

A stacking facility may be open, covered, or roofed and is used for wastes which behave primarily as solid. Determine the wall height using the anticipated stacking angle of the waste material. Construct a stacking facility of durable materials such as reinforced concrete, reinforced concrete block, or treated lumber. Design the stacking facility with adequate safety factors to prevent failure due to internal or external pressures, including hydrostatic uplift pressure and imposed surface loads such as equipment which may be used within, on, or adjacent to the structure.

Foundation. The finished bottom of elevation of the stacking facility shall be a minimum of 2 feet above the seasonal high water table unless special design features are incorporated that address buoyant forces, impoundment seepage rate and non-encroachment of the water table by contaminants. The water table may be lowered by use of drains to meet this requirement. Locate the bottom elevation a minimum of 2 feet above bedrock regardless of lining type. Blasting of bedrock within 300 feet of an on-farm water source, i.e. well, spring, etc., must have State Conservation Engineer's approval.

Seepage. Prevent leachate in amounts that would pollute surface or groundwater with collection and disposal of liquids in a safe manner as necessary. Prevent influent seepage in amounts that would infringe on designed storage capacity. Seepage control may not be necessary on sites that have a roof, waste material with little seepage potential or in certain climates.

Internal Drainage. Make provisions for drainage of leachate, including rainfall from the stacking area (especially those without a roof). Collect leachate in a tank or waste storage facility, or properly treat in a lagoon or vegetated treatment area.

Poultry Litter Stacking Facility. To reduce the potential for spontaneous combustion damage to wood walled facilities, design the height of the litter stack not to exceed 7 feet, with litter to wood contact limited to 5 feet.

Bedded Pack. Bedded pack facilities shall be heavily bedded to adequately support the livestock and so the livestock are kept clean. The moisture content of the pack shall be maintained at 55 to 65 percent or drier. The livestock space requirement for bedded pack facilities shall not exceed 100 square feet per animal unit.

CONSIDERATIONS

For exposed liners utilizing HDPE or similar materials that are slippery when wet, consider the use of textured liners or addition of features such as tire ladders that would allow for escape from the waste storage structure.

Consider solid/liquid separation of runoff or wastewater entering impoundments to minimize the frequency of accumulated solids removal and to facilitate pumping and application of the stored waste.

Due consideration should be given to environmental concerns, economics, the overall waste management system plan, and safety and health factors.

Since the economics and risks associated with waste storage facilities are quite high, consider providing the operator with the cost to close the facility. Cost should include removal of the planned sludge accumulation volume and the waste stored at the maximum operating volume.

When possible, install sump areas, up to two feet lower than the design bottom, to facilitate emptying the facility.

Considerations for Siting

Consider the following factors in selecting a site for waste storage facilities:

- Proximity of the waste storage facility to the source of waste.
- Access to other facilities.
- Ease of loading and unloading waste.
- Compatibility with the existing landforms and vegetation, including building arrangement, to minimize odors and adverse impacts on visual resources.
- Adequate maneuvering space for operating, loading, and unloading equipment.

Considerations for Minimizing the Potential for and Impacts of Sudden Breach of Embankment or Accidental Release from the Waste Storage Facility.

Consider features, safeguards, and/or management measures to minimize the risk of failure or accidental release, or to minimize or mitigate impact of this type of failure when any of the categories listed below might be significantly affected.

Potential impact categories from breach of embankment or accidental release include—

- Surface water bodies—perennial streams, lakes, wetlands, and estuaries.
- Critical habitat for threatened and endangered species.
- Riparian areas.
- Farmstead, or other areas of habitation.
- Off-farm property

- Historical and archaeological sites or structures that meet the eligibility criteria for listing in the National Register of Historical Places.

Consider the following either singly or in combination to minimize the potential of or the consequences of sudden breach of embankments:

- An auxiliary (emergency) spillway.
- Additional freeboard.
- Storage for wet year rather than normal year precipitation.
- Reinforced embankment— such as, additional top width, flattened and/or armored downstream side slopes.
- Secondary containment.
- Double liners.

Options to consider to minimize the potential for accidental release from the waste storage facility through gravity outlets include—

- Outlet gate locks or locked gate housing.
- Secondary containment.
- Alarm system.
- Another nongravity means of emptying the waste storage facility.

Considerations for Minimizing the Potential of Waste Storage Pond Liner Failure.

Avoid sites with categories listed below unless no reasonable alternative exists.

Potential impact categories for liner failure are—

- Any underlying aquifer is at a shallow depth and not confined.
- The vadose zone is rock.
- The aquifer is a domestic water supply or ecologically vital water supply.
- The site is located in an area of water soluble bedrock such as limestone or gypsum.

For a site with one or more of these site conditions, consider providing a leak detection system in conjunction with the planned liner to provide an additional measure of safety.

Considerations for Stacking Facilities

Internal seepage collection within a stacking facility can be accomplished by use of a timber wall with the boards installed vertically, leaving 3/4-inch cracks. The timber wall drainage section may be included in a concrete or masonry block wall. Use the design criteria for timber walls.

For any facility that is an organic producer or that sells manure to organic producers, consider using rot-resistant or treated lumber that meets the requirements for organic production. The producer should consult with the organic certifier as to the use and acceptability of treated lumber for waste storage.

Considerations for Improving Air Quality

Liquid manure storage may result in emissions of volatile organic compounds, ammonia, hydrogen sulfide, methane, nitrous oxide, and carbon dioxide. Solid manure storage may result in emissions of particulate matter, volatile organic compounds, ammonia, carbon dioxide, and nitrous oxide.

To reduce emissions of greenhouse gases, ammonia, volatile organic compounds, particulate matter and odor, other NRCS CPSs such as Anaerobic Digester (Code 366), Roofs and Covers (Code 367), Waste Treatment (Code 629), Amendments for Treatment of Agricultural Waste (Code 591), Composting Facility (Code 317), and Air Filtration and Scrubbing (Code 371) can be added to the waste management system.

Adjusting pH below 7 may reduce ammonia emissions from the waste storage facility but may increase odor when waste is surface applied—see NRCS CPS Nutrient Management (Code 590).

Some fabric and organic covers have been shown to be effective in reducing odors.

Maintain appropriate manure moisture content for solid manure storage facilities. Excessive moisture will increase the potential for air emissions of volatile organic compounds, ammonia, and nitrous oxide, and may lead to anaerobic conditions, which will increase the potential for emissions of methane and hydrogen sulfide. Too little moisture will increase the potential for particulate matter emissions.

PLANS AND SPECIFICATIONS

Prepare plans and specifications that describe the requirements for applying the practice to achieve its intended use. As a minimum, include the following in the engineering plans and specifications:

- Plan view of system layout.
- Structural details of all components, including reinforcing steel, type of materials, thickness, anchorage requirements, lift thickness.
- Locations, sizes, and type of pipelines and appurtenances.
- Requirements for foundation and preparation and treatment.
- Vegetative requirements.
- Quantities.
- Approximate location of utilities and notification requirements.

OPERATION AND MAINTENANCE

Develop an operation and maintenance plan that is consistent with the purposes of the practice, its intended life, safety requirements, and the criteria for its design. At a minimum, the plan will contain where appropriate:

[Discuss the operational and maintenance needs with the landowner or operator who is responsible for the practice. Bring to the responsible person's attention all potential hazards associated with the waste storage facility. Prior to construction, provide copies of the O&M plan to the owner/operator. Have the owner/operator sign the O&M plan to indicate an understanding of the requirements and a commitment to operation and maintenance of the waste storage facility.](#)

Include the operational requirements for emptying the storage facility including the expected storage period. Begin removal of the liquid storage facility as soon as practical after the maximum operating level has been reached. Also include the requirement that waste be removed from storage and utilized at locations, times, rates, and volume in accordance with the overall waste management system plan.

For impoundments and other liquid storages include an explanation of the staff gauge or other permanent marker to indicate the maximum operating level. For storages where the contents are not visible and a staff gauge would not be visible, such as below a slatted floor, identify the method for the operator to measure the depth of accumulated waste.

Include a provision for emergency removal and disposition of liquid waste in the event of an unusual storm event that may cause the waste storage structure to fill to capacity prematurely.

Include instructions as needed for ventilating confined spaces according to ASABE Standard S607, Venting Manure Storages to Reduce Entry Risk.

Develop an emergency action plan for waste storage facilities where there is a potential for significant impact from breach or accidental release. Include site-specific provisions for emergency actions that will minimize these impacts.

Include a description of the routine maintenance needed for each component of the facility. Also include provisions for maintenance that may be needed as a result of waste removal or material deterioration.

REFERENCES

American Society for Testing and Materials. Annual Book of ASTM Standards. Standards D 653, D 698, D 1760, D 2488. ASTM, Philadelphia, PA.

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